



# *APPENDIX B*

## **Origin-Destination Study**

## ***APPENDIX B : ORIGIN – DESTINATION STUDY***

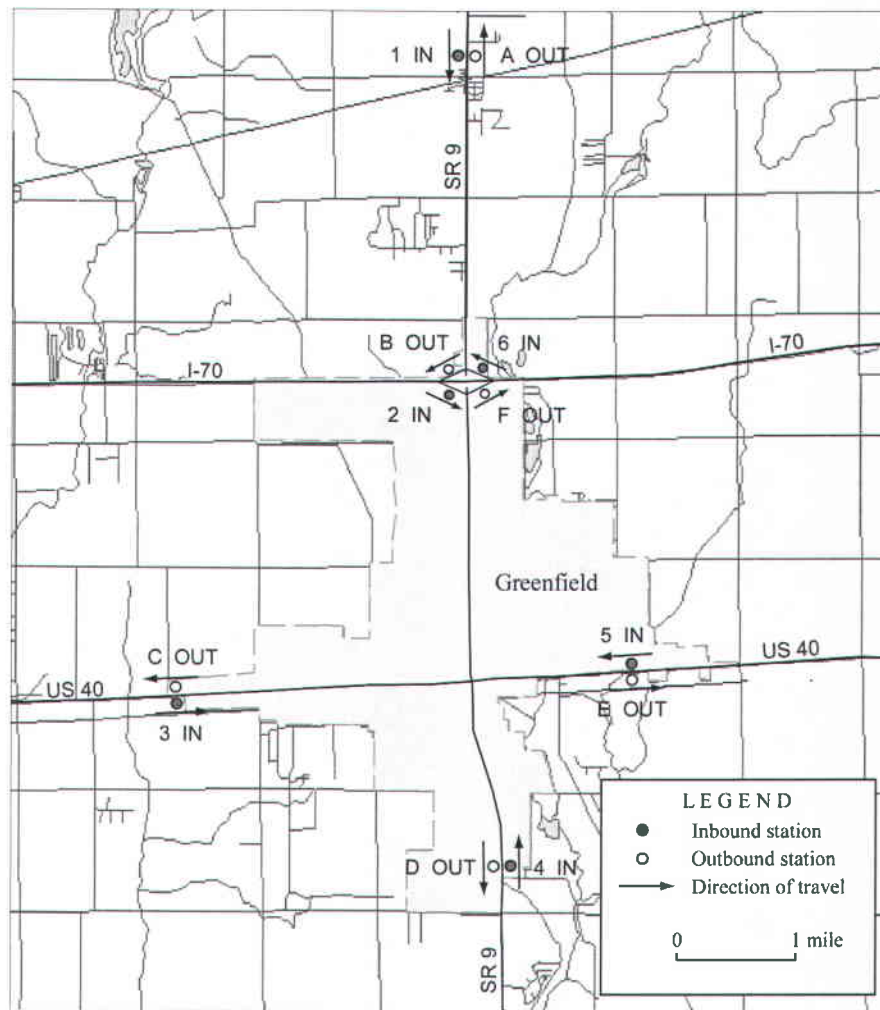
### ***DATA COLLECTION***

Existing weekday through trip data were collected through a cooperative effort by Paul I. Cripe Inc. (PIC) and Pflum, Klausmeier & Gehrum Consultants, Inc. (PKG) to conduct a manual license plate survey at twelve one-directional observation points on the periphery of the SR 9 arterial corridor serving the City of Greenfield.

Figure A.01 shows the location of the survey stations. As indicated on the map, stations designated 1 through 6 carry traffic into the Greenfield study area; stations designated A through F carry traffic away from the study area.

FIGURE A.01 : License Plate Survey Stations





Continuous and coordinated license plate data were recorded during three 2.5-hour periods on Wednesday, August 22, 2001. During two periods of peak local traffic volumes, 6:00 AM to 8:30 AM and 3:30 PM to 6:00 PM, observers sequentially recorded all legible license plates of passenger vehicles and tallied the number of heavy vehicles, whether trucks or busses. A third midday observation period from 11:00 AM to 1:30 PM was used for recording license plates of heavy vehicles only. Anticipated difficulty in reading the license plates of heavy vehicles during periods of low-angle sunlight and high traffic volumes was the primary reason for restricting recording of these vehicles to the midday off-peak period.

Using voice recorders, observers verbally recorded the last four license plate characters of each passing vehicle. On the afternoon of the day preceding the survey, observers were briefed on the variety of license plate types to anticipate, including a uniform procedure for reporting plates with less than four digits. Using synchronized watches, observers also reported times of day within the stream of license plate data at fifteen-minute intervals.

Survey personnel were instructed to record entries of "T" (truck) or "B" (bus) for heavy vehicles because of the perceived difficulty in identifying and reading license plates of such vehicles when interspersed in high volumes of traffic. The survey processing procedure acknowledges that these data are invalid for the purpose of determining matches within sequential data streams and requires such records to be deleted after being tabulated.

All audio recording cassettes were labeled with location, observation period, and sequence of use information.

## ***DATA PREPROCESSING***

The audio recordings of license plate data and periodic time observations were transcribed into Microsoft Excel worksheets. Transcriptions were checked for consistency of entry during preprocessing steps prior to the license plate matching procedure programmed to extract the information used in the analysis described in this chapter.

Periodic time observations in the worksheets were converted to a uniform timeline expressed in seconds from the synchronized beginning of each 2.5-hour survey period.

By using [2.5 hours = 150 minutes = 9000 seconds] as a conversion factor, time of day notations were expressed as values between 0 (beginning of survey) and 9000 (end of survey). Sequential license plate records between time notations were then assigned linear time values in proportion to the number of entries between time values. All time values were expressed to the nearest second.

It is noted that this assignment relies upon a working assumption that the distribution of vehicles between time points would be relatively uniform. This assumption also depends upon a reasonable frequency of time observations rather than the absolute accuracy of individual observations.

Before the time values were calculated for the listing of passenger vehicle license plates during the AM and PM surveys, each sequential list was screened for the presence of entries indicating heavy vehicles (T=truck, B=bus) or passenger vehicles with unreadable plates (A=auto, C=car, M=motorcycle). As stated previously, such records were removed from all data input lists prior to the matching procedure. Table A.01 shows the number of transcribed license plates and the number of excluded records for each of the twelve stations during the AM and PM surveys.



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Table A.01 also shows that the data transcriptions contained small numbers of entries which could not be successfully translated when the Excel worksheets were converted to .dbf format database tables. These translation failures are attributed to worksheet entries returning invalid cell addresses, e.g. "-N86", where a right aligned three character text entry had been attended. The number of such invalid records, when present, is small.

TABLE A.01 : License Plate Data Preprocessing Summary

Station Filename	Transcribed Records	B	T	A	M	C	X	Net * Plates	Percent Valid Records
AM 1 in	796	17	1	0	0	0	2	776	97.5%
AM 2 in	488	0	80	3	0	0	1	404	82.8%
AM 3 in	1151	4	63	33	0	0	2	1049	91.1%
AM 4 in	874	1	98	2	1	0	2	770	88.1%
AM 5 in	943	1	76	16	0	3	0	847	89.8%
AM 6 in	215	0	23	0	0	0	0	192	89.3%
AM A out	713	1	58	0	0	0	4	650	91.2%
AM B out	2332	0	123	41	0	2	4	2162	92.7%
AM C out	1081	2	12	7	1	0	4	1055	97.6%
AM D out	565	0	110	71	2	1	3	378	66.9%
AM E out	456	0	39	11	0	0	0	406	89.0%
AM F out	334	0	21	4	0	0	1	308	92.2%
PM 1 in	769	4	0	0	0	0	4	761	99.0%
PM 2 in	1880	0	71	85	0	0	3	1721	91.5%
PM 3 in	1788	1	39	71	0	0	0	1677	93.8%
PM 4 in	846	0	82	21	0	0	3	740	87.5%
PM 5 in	737	1	36	25	0	0	0	675	91.6%
PM 6 in	378	0	36	8	0	0	4	330	87.3%
PM A out	1323	14	88	25	0	0	3	1193	90.2%
PM B out	1083	0	106	49	0	0	2	926	85.5%
PM C out	1576	1	36	51	0	0	4	1484	94.2%
PM D out	913	0	97	30	0	0	2	784	85.9%
PM E out	641	0	1	76	0	0	0	564	88.0%
PM F out	469	0	4	24	0	0	0	441	94.0%

B= School bus [ license plate not recorded ]

T= Truck [ license plate not recorded ]

A= Automobile [ license plate illegible ]

M= Motorcycle [ license plate illegible ]

C= Car [auto] [license plate illegible ]

X= Invalid plate transcription

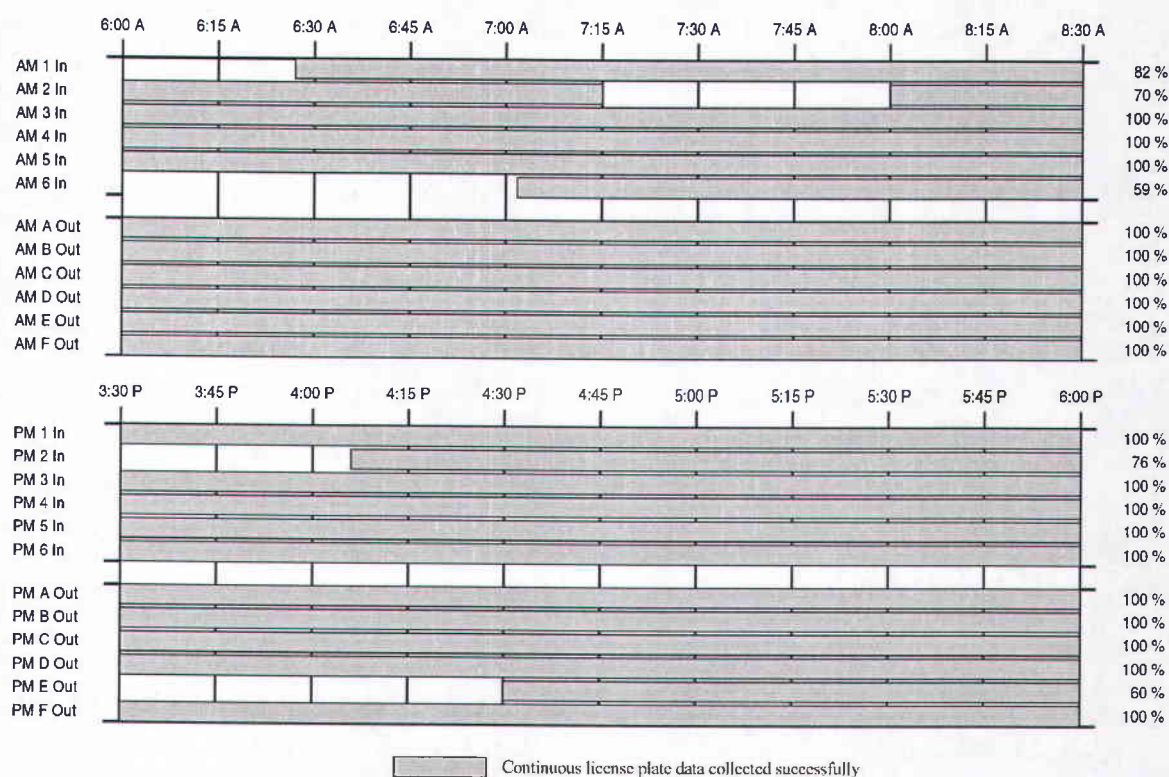
\* Net plates = Transcribed Records less [ B, T, A, M, C, X ]



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During the transcription and data preprocessing tasks, several instances of recording lapses were taken into account. These lapses are attributed to personnel error, e.g. failure to properly activate the voice recorder. The presence of periodic time points in the unaffected portions of each recording supported extrapolation of linear time lines to all records. Figure A.02 shows the occurrence and duration of the recording lapses and indicates the amount of usable data as a percentage of total survey time. Evaluation of these lapses suggests that the survey data were not overly compromised.

FIGURE A.02 : Occurrence and Duration of Survey Recording Gaps



## ***LICENSE PLATE MATCHING PROCEDURE***

The synchronized license plate survey was designed to detect the number of through vehicular trips using the SR 9 corridor in the Greenfield area. Through trips are made by vehicles entering the corridor at one of the inbound stations [1-6] and exiting the corridor at one of the outbound stations [A-F].

Knowledge of the usual driving time to traverse the SR 9 corridor during the survey period is essential in determining which trips are, in fact, through trips. Simple matching of license plates detected at two different stations in a survey lasting 2.5 hours is insufficient for this determination. A specific license plate at an inbound station must not only be identified at an outbound station; it must also have occurred with an appropriate time interval between observations.

For example, a vehicle entering at Station 2 IN (eastbound I-70 exit ramp at SR 9) and exiting at Station D OUT (southbound SR 9 at the southern limits of Greenfield) must have sufficient time to travel 4.2 miles in the stream of traffic using SR 9. However, the time allowed for this action must not be so great that intermediate stops may have been made.

For the purpose of this survey, allowable ranges of driving time between inbound and outbound stations were estimated by repeated travel across the study area. Minimum and maximum times for each trip interchange are displayed in Table A.02 with times expressed both in minutes and seconds.

The matrix of trip interchanges in this table shows inbound (origin) stations along the left-hand side and outbound (destination) stations along the top. This organization will be used in the remainder of this chapter. Moreover, the matrix is one-directional; that is, origin-destination pairs always begin at an inbound (1-6) station and end at an outbound station (A-F).



TABLE A.02 : Minimum / Maximum Station to Station Drive Times

TIME IN MINUTES		Outbound					
		A	B	C	D	E	F
Inbound	1		2/6	12/20	12/20	12/20	2/6
	2	2/6		10/17	10/17	10/17	
	3	12/20	10/17		9/14	7/13	9/16
	4	12/20	10/17	9/14		7/13	10/17
	5	12/20	10/17	7/13	7/13		9/16
	6	2/6		9/16	10/17	9/16	

TIME IN SECONDS		Outbound					
		A	B	C	D	E	F
Inbound	1		120/360	720/1200	720/1200	720/1200	120/360
	2	120/360		600/1020	600/1020	600/1020	
	3	720/1200	600/1020		540/840	420/780	540/960
	4	720/1200	600/1020	540/840		420/780	600/1020
	5	720/1200	600/1020	420/780	420/780		540/960
	6	120/360		540/960	600/1020	540/960	

By definition, through trips must begin and end at different locations relative to the SR 9 corridor; therefore, comparisons between inbound and outbound data at the same geographic location were not made; e.g. license plates from station 1 IN are not compared to plates recorded at A OUT. Origin-destination pairs which do not meet this through trip definition are shaded and have no values.

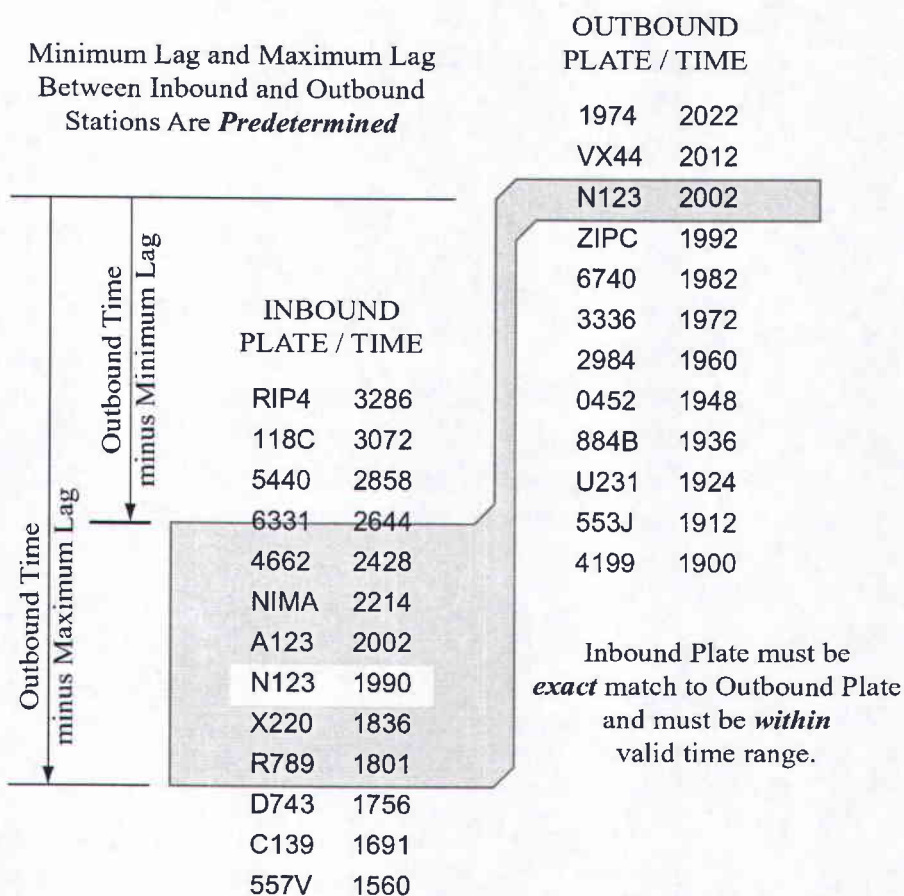
Database files developed by the preprocessing steps consist of one record per recorded license plate containing the four characters and a sequential time value. Files were named for the survey period and station information contained in the file, e.g. AM\_1\_IN or PM A\_OUT.

The plate matching algorithm was implemented in a series of routines in Microsoft FoxPro database management software. A single illustrative description will be used in the paragraphs that follow. All origin-destination pairs were subjected to the same straightforward procedure.

Because the through trip origin-destination pairs are one-directional and time synchronized, the matching algorithm selected for this procedure takes a specified outbound station, for example AM A OUT and reads the minimum and maximum time values established for trips from a single inbound list. Inbound AM\_2\_IN is the first applicable inbound list for AM\_A\_OUT. The program sequentially reads each plate record in the AM\_A\_OUT file and its corresponding time value (elapsed time in seconds from the beginning of the survey) and calculates the minimum and maximum allowable times for a potential inbound observation to have occurred. This algorithm is schematically illustrated by Figure A.03.



FIGURE A.03 : License Plate Matching Validation Parameters



If a plate record in the AM\_2\_IN file is found, and if that the time value of that record falls within the established range, then a copy of that record is written to a third file containing 2-to-A through trip records. If no time eligible match is found, no record is written. The program then reads the next record in AM\_A\_OUT and repeats the comparison-write/no-write subroutine, continuing until all records in AM\_A\_OUT have been compared against all records in AM\_2\_IN.

The program then performs a similar comparison between all records in AM\_A\_OUT and records in AM\_3\_IN and continues until AM\_4\_IN, AM\_5\_IN, and AM\_6\_IN are similarly examined. The entire process is repeated in turn for each outbound station, with each repetition generating a file of origin-destination matches identified by survey period and stations.

The number of records in each of the output files equals the number of time constrained matching license plates between one specified inbound station and one specified outbound station. By the design of the matching routine, the number of records is the number of through trips from that station to station pair. Table A.03 displays the number of raw AM and PM through trip matches in the matrix format introduced above.

TABLE A.03 : Raw AM and PM Through Trip Matches

Raw AM Matches		Outbound					
		A	B	C	D	E	F
Inbound	1		94	10	9	4	11
	2	16		1	0	1	
	3	10	7		2	20	3
	4	25	36	11		7	3
	5	6	26	44	6		2
	6	6		1	0	1	

Raw PM Matches		Outbound					
		A	B	C	D	E	F
Inbound	1		57	11	27	2	8
	2	164		14	20	16	
	3	13	7		12	58	5
	4	73	8	7		2	8
	5	10	10	47	4		1
	6	21		4	2	0	

## ADJUSTMENTS TO THE RAW THROUGH TRIP DATA

After tabulation of the AM and PM matching routines, two procedural adjustments were made to the raw through trip data. One adjustment was made to account for (1) missing data associated with gaps in survey recording [See Figure A.02]. The other adjustment reconciled minor discrepancies between the net number of transcribed license plates [See Table A.01] and the number of vehicles detected by INDOT mechanical vehicle counts during the day of the survey.

The time adjustment was made with the assumption that the average frequency of through trips per unit of time during the recording gaps would be equivalent to the average frequency of through trips during the remainder of the recording. This adjustment is accomplished by a simple extrapolation of each cell in the raw match matrix from the actual number of recording seconds achieved during each station to station interchanges. Table A.04 illustrates the results of this extrapolation for the AM and PM surveys, respectively.

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The calculations to extrapolate time adjusted matches multiply the raw match cell entry by a factor of 9000 potential survey seconds divided by the actual number of recorded seconds available to the special cell, for example :

Station 1 to Station B = 94;  $94 \times (9000 / 7380) = 115$  rounded to an integer

Station 4 to Station A = 25;  $25 \times (9000 / 9000) = 25$  rounded to an integer

TABLE A.04 : Calculation of Time Adjusted AM and PM Matches

Raw AM Matches		Outbound					
		A	B	C	D	E	F
Inbound	1		94	10	9	4	11
	2	16		1	0	1	
	3	10	7		2	20	3
	4	25	36	11		7	3
	5	6	26	44	6		2
	6	6		1	0	1	

Raw PM Matches		Outbound					
		A	B	C	D	E	F
Inbound	1		57	11	27	2	8
	2	164		14	20	16	
	3	13	7		12	58	5
	4	73	8	7		2	8
	5	10	10	47	4		1
	6	21		4	2	0	

AM Seconds		Outbound					
		A	B	C	D	E	F
Inbound	1		7380	7380	7380	7380	7380
	2	6300		6300	6300	6300	
	3	9000	9000		9000	9000	9000
	4	9000	9000	9000		9000	9000
	5	9000	9000	9000	9000		9000
	6	5280		5280	5280	5280	

PM Seconds		Outbound					
		A	B	C	D	E	F
Inbound	1		9000	9000	9000	5400	9000
	2	6840		6840	6840	5400	
	3	9000	9000		9000	5400	9000
	4	9300	9300	9300		5400	9300
	5	9000	9000	9000	9000		9000
	6	9000		9000	9000	5400	

Time Adjusted AM Matches		Outbound					
		A	B	C	D	E	F
Inbound	1		115	12	11	5	13
	2	23		1	0	1	
	3	10	7		2	20	3
	4	25	36	11		7	3
	5	6	26	44	6		2
	6	10		2	0	2	

Time Adjusted PM Matches		Outbound					
		A	B	C	D	E	F
Inbound	1		57	11	27	3	8
	2	216		18	26	27	
	3	13	7		12	97	5
	4	71	8	7		3	8
	5	10	10	47	4		1
	6	21		4	2	0	

Table A.04 also displays the results of a secondary expansion of the time adjusted matches to account for differences between the net number of usable vehicle plates at each license plate survey station as compared to the total number of vehicles mechanically counted at the same station.

A secondary expansion of the time adjusted matches was then made to proportionally balance the matrix of through trips after expanding row and column control totals to correspond to the total of vehicles reported at each station during the license plate survey. Although mathematically straightforward, the rationale behind this expansion requires a brief explanation.



The sum of values in each row of the time adjusted AM or PM matrix represents the total number of matches at each inbound survey station. The sum of values in each column of the same matrix represents the total number of matches at each outbound station. The matrix is balanced because the values in its cell contribute to both row and column totals.

There is an implicit relationship between the number of through trips passing each survey station and the total volume of vehicles passing the same station. Recall that in Table A.01 the total number of transcribed records was reduced by the number of heavy vehicles, passenger vehicles with unreadable plates, and miscellaneous invalid transcriptions. The matrix of time adjusted matches was systematically developed from the net number of usable plates, not from the total number of plates.

Time adjusted matrix row totals were expanded in proportion to the ratio between the total number of inbound survey vehicles divided by the net number of inbound plates at a station. Matrix column totals were similarly expanded by the ratio between total outbound survey vehicles and net outbound plates at each station.

Differences between the time adjusted matrix row and column control totals and the expanded totals were then distributed to matrix cells proportionally to the ratio between a cell's time adjusted value and its percentage of the row or column total. Calculations of this volume expansion were constrained to integer values (unit vehicles), so a portion of the calculations involving very small differences or very small cell values did not produce a complete unit value.

The results of this volume based expansion are shown in Table A.05. Matrix values in this table are the basis for all subsequent adjustments of the license plate survey data to incorporate INDOT mechanical vehicle counts and vehicle classification counts.





TABLE A.05 : Volume Expanded AM and PM Through Trips

Volume Expanded AM Through		Outbound					
		A	B	C	D	E	F
Inbound	1		121	12	14	6	14
	2	27		1	0	1	
	3	11	8		3	22	3
	4	28	40	12		8	3
	5	7	29	47	8		2
	6	11		2	0	2	

Volume Expanded PM Through		Outbound					
		A	B	C	D	E	F
Inbound	1		63	12	30	3	8
	2	238		20	29	31	
	3	14	8		14	107	5
	4	80	9	8		3	9
	5	11	12	51	5		1
	6	24		5	2	0	

Calculations of the expanded control totals defined above are summarized in Table A.06 on the following page. Note that this table is based on the data presented in Table A.01.

For the purpose of expanding the control totals, heavy vehicles (truck or bus) were considered to be vehicles undifferentiated from passenger vehicles within the traffic streams monitored during the license plate survey.

Later portions of this chapter will document the need and methods used to differentiate heavy vehicles from passenger vehicles. An initial assumption that the distribution of heavy vehicles at individual survey stations would be similar to the distribution of passenger vehicles was detected during analysis of vehicle classification data. Awareness of the different impacts from the two types of vehicles on local traffic flow will contribute to the understanding of the remainder of this chapter.

Awareness of the perception of heavy vehicles on traffic flow will be equally important to other portions of this report.

TABLE A.06 : Calculation of Station Volume Based Expansion of Through Trips

Filename	Transcribed Records	Net * Plates	Time Adjusted Matched Plates	Volume Adjusted Through Trips	Difference	Logged Seconds	Time Adjusted Total Trips
AM 1 in	796	776	156	160	4	7380	971
AM 2 in	488	404	25	30	5	6300	697
AM 3 in	1151	1049	42	46	4	9000	1151
AM 4 in	874	770	82	93	11	9000	874
AM 5 in	943	847	84	94	10	9000	943
AM 6 in	215	192	14	16	2	5280	366
AM A out	713	650	74	81	7	9000	713
AM B out	2332	2162	184	198	14	9000	2332
AM C out	1081	1055	70	72	2	9000	1081
AM D out	565	378	19	28	9	9000	565
AM E out	456	406	35	39	4	9000	456
AM F out	334	308	21	23	2	9000	334
PM 1 in	769	761	106	107	1	9000	769
PM 2 in	1880	1721	287	314	27	6840	2474
PM 3 in	1788	1677	134	143	9	9000	1788
PM 4 in	846	740	97	111	14	9300	819
PM 5 in	737	675	72	79	7	9000	737
PM 6 in	378	330	27	31	4	9000	378
PM A out	1323	1193	331	367	36	9000	1323
PM B out	1083	926	82	96	14	9000	1083
PM C out	1576	1484	87	92	5	9000	1576
PM D out	913	784	71	83	12	9000	913
PM E out	641	564	130	148	18	5400	1068
PM F out	469	441	22	23	1	9000	469

Table A.06 also shows the time adjusted total trips at each survey station. This adjustment was a simple extrapolation of the total number of transcribed records at each station, as was done independently for matching through trips. Again, the time adjustment was based on the assumption that the average frequency of vehicles passing a station during gaps in the recordings would be equivalent to the average frequency during the remainder of the recording.

The extrapolation multiplies the number of transcribed records by a factor of 9000 potential survey seconds divided by the actual number of logged recording seconds, for example :

Station AM 1 IN = 796 ;  $796 \times (9000 / 7380) = 971$  rounded to an integer

Station PM 1 IN = 769 ;  $769 \times (9000 / 9000) = 769$  rounded to an integer



## SUMMARY OF AM AND PM LICENSE PLATE SURVEY RESULTS

Tables A.07 (a) and (b) show the adjusted results of the 2.5 hour license plate surveys during morning and afternoon periods of peak traffic volumes along the SR 9 corridor in the Greenfield vicinity. Each table is organized to show through trips as components of the traffic streams entering the corridor at inbound survey stations and also as components of the traffic streams leaving the corridor at outbound survey stations.

Table A.07 (a) : Through Vehicle Trip Analysis – 6:00 AM to 8:30 PM, August 22, 2001

Enter at 1 : 971	Exit at B :	Volume	Percent	Exit at A : 713	Enter at 2 :	Volume	Percent
	Exit at C :	101	12.5%		Enter at 3 :	27	3.8%
	Exit at D :	12	1.2%		Enter at 4 :	11	1.5%
	Exit at E :	14	1.4%		Enter at 5 :	28	3.9%
	Exit at F :	6	0.6%		Enter at 6 :	7	1.0%
	Exit at F :	14	1.4%		Enter at 6 :	11	1.5%
Total Through:		167	17.2%	Total Through:		84	11.8%
To Other :		804	82.8%	From Other :		629	88.2%
Total :		971	100.0%	Total :		713	100.0%
Enter at 2 : 697	Exit at A :	Volume	Percent	Exit at B : 2332	Enter at 1 :	Volume	Percent
	Exit at C :	27	3.9%		Enter at 3 :	121	5.2%
	Exit at D :	1	0.1%		Enter at 4 :	8	0.3%
	Exit at E :	0	0.0%		Enter at 5 :	40	1.7%
	Exit at E :	1	0.1%		Enter at 5 :	29	1.2%
	Exit at E :	1	0.1%		Enter at 5 :	29	1.2%
Total Through:		29	4.2%	Total Through:		198	8.5%
To Other :		668	95.8%	From Other :		2134	91.5%
Total :		697	100.0%	Total :		2332	100.0%
Enter at 3 : 1151	Exit at A :	Volume	Percent	Exit at C : 1081	Enter at 1 :	Volume	Percent
	Exit at B :	11	1.0%		Enter at 2 :	12	1.1%
	Exit at D :	8	0.7%		Enter at 3 :	1	0.1%
	Exit at E :	3	0.3%		Enter at 4 :	12	1.1%
	Exit at E :	22	1.9%		Enter at 5 :	47	4.3%
	Exit at F :	3	0.3%		Enter at 6 :	2	0.2%
Total Through:		47	4.1%	Total Through:		74	6.8%
To Other :		1104	95.9%	From Other :		1007	93.2%
Total :		1151	100.0%	Total :		1081	100.0%
Enter at 4 : 874	Exit at A :	Volume	Percent	Exit at D : 565	Enter at 1 :	Volume	Percent
	Exit at B :	28	3.2%		Enter at 2 :	14	2.5%
	Exit at C :	40	4.6%		Enter at 3 :	0	0.0%
	Exit at E :	12	1.4%		Enter at 4 :	3	0.5%
	Exit at E :	8	0.9%		Enter at 5 :	8	1.4%
	Exit at F :	3	0.3%		Enter at 6 :	0	0.0%
Total Through:		91	10.4%	Total Through:		25	4.4%
To Other :		783	89.6%	From Other :		540	95.6%
Total :		874	100.0%	Total :		565	100.0%
Enter at 5 : 943	Exit at A :	Volume	Percent	Exit at E : 456	Enter at 1 :	Volume	Percent
	Exit at B :	7	0.7%		Enter at 2 :	6	1.3%
	Exit at C :	29	3.1%		Enter at 3 :	1	0.2%
	Exit at D :	47	5.0%		Enter at 4 :	32	4.8%
	Exit at E :	6	0.6%		Enter at 5 :	8	1.8%
	Exit at F :	2	0.2%		Enter at 6 :	2	0.4%
Total Through:		93	9.9%	Total Through:		39	8.6%
To Other :		850	90.1%	From Other :		417	91.4%
Total :		943	100.0%	Total :		456	100.0%
Enter at 6 : 366	Exit at A :	Volume	Percent	Exit at F : 334	Enter at 1 :	Volume	Percent
	Exit at B :	11	3.0%		Enter at 2 :	14	4.2%
	Exit at C :	2	0.5%		Enter at 3 :	3	0.9%
	Exit at D :	0	0.0%		Enter at 4 :	3	0.9%
	Exit at E :	2	0.5%		Enter at 5 :	2	0.6%
	Exit at E :	2	0.5%		Enter at 5 :	2	0.6%
Total Through:		15	4.1%	Total Through:		22	6.6%
To Other :		351	95.9%	From Other :		312	93.4%
Total :		366	100.0%	Total :		334	100.0%



Table A.07 (b) : Through Vehicle Trip Analysis – 3:30 PM to 6:00 PM, August 22, 2001

Enter at 1 : 769	Volume Percent		Exit at A : 1323	Volume Percent	
	Exit at B :	63 8.2%		Enter at 2 :	238 18.0%
	Exit at C :	12 1.6%		Enter at 3 :	14 1.1%
	Exit at D :	30 3.9%		Enter at 4 :	80 6.0%
	Exit at E :	3 0.4%		Enter at 5 :	11 0.8%
	Exit at F :	8 1.0%		Enter at 6 :	24 1.8%
	Total Through:	116 15.1%		Total Through:	367 27.7%
		To Other :	653 84.9%	From Other :	
		Total :	769 100.0%	Total :	
Enter at 2 : 2474	Volume Percent		Exit at B : 1083	Volume Percent	
	Exit at A :	238 9.6%		Enter at 1 :	63 5.8%
	Exit at C :	20 0.8%		Enter at 3 :	8 0.7%
	Exit at D :	29 1.2%		Enter at 4 :	9 0.8%
	Exit at E :	31 1.3%		Enter at 5 :	12 1.1%
	Total Through:	318 12.9%		Total Through:	92 8.5%
	To Other :	2156 87.1%		From Other :	991 91.5%
		Total :	2474 100.0%	Total :	
Enter at 3 : 1788	Volume Percent		Exit at C : 1576	Volume Percent	
	Exit at A :	14 0.8%		Enter at 1 :	12 0.8%
	Exit at B :	8 0.4%		Enter at 2 :	20 1.3%
	Exit at D :	14 0.8%		Enter at 4 :	8 0.5%
	Exit at E :	107 6.0%		Enter at 5 :	51 3.2%
	Exit at F :	5 0.3%		Enter at 6 :	5 0.3%
	Total Through:	148 8.3%		Total Through:	96 6.1%
		To Other :	1640 91.7%	From Other :	
		Total :	1788 100.0%	Total :	
Enter at 4 : 819	Volume Percent		Exit at D : 913	Volume Percent	
	Exit at A :	80 9.8%		Enter at 1 :	30 3.3%
	Exit at B :	9 1.1%		Enter at 2 :	29 3.2%
	Exit at C :	8 1.0%		Enter at 3 :	14 1.5%
	Exit at E :	3 0.4%		Enter at 5 :	5 0.5%
	Exit at F :	9 1.1%		Enter at 6 :	2 0.2%
	Total Through:	109 13.3%		Total Through:	80 8.8%
		To Other :	710 86.7%	From Other :	
		Total :	819 100.0%	Total :	
Enter at 5 : 737	Volume Percent		Exit at E : 1068	Volume Percent	
	Exit at A :	11 1.5%		Enter at 1 :	3 0.3%
	Exit at B :	12 1.6%		Enter at 2 :	31 2.9%
	Exit at C :	51 6.9%		Enter at 3 :	107 10.0%
	Exit at D :	5 0.7%		Enter at 4 :	3 0.3%
	Exit at F :	1 0.1%		Enter at 6 :	0 0.0%
	Total Through:	80 10.9%		Total Through:	144 13.5%
		To Other :	657 89.1%	From Other :	
		Total :	737 100.0%	Total :	
Enter at 6 : 378	Volume Percent		Exit at F : 469	Volume Percent	
	Exit at A :	24 6.3%		Enter at 1 :	8 1.7%
	Exit at C :	5 1.3%		Enter at 3 :	5 1.1%
	Exit at D :	2 0.5%		Enter at 4 :	9 1.9%
	Exit at E :	0 0.0%		Enter at 5 :	1 0.2%
	Total Through:	31 8.2%		Total Through:	23 4.9%
	To Other :	347 91.8%		From Other :	446 95.1%
		Total :	378 100.0%	Total :	

In Tables A.07 (a) and (b) data representing the total volume of traffic at each station consist of the adjusted total number of transcribed vehicle records. Following sections of this chapter compare these volumes to INDOT mechanical vehicle counts and vehicle classification counts.

## ***DISCUSSION OF MID-DAY HEAVY VEHICLE LICENSE PLATE SURVEY***

As stated in the Data Collection section at the beginning of this chapter, a mid-day survey of heavy vehicles using the SR 9 corridor was conducted between the hours of 11:00 AM and 1:30 PM. During this 2.5 hour survey, observers recorded the identifiable and readable license plates of heavy (truck and bus) vehicles only. Passenger vehicles were neither recorded or counted.

Uncertainty about the difficulty and time required to read and record license plates on heavy vehicles and concerns about personnel fatigue contributed to the decision to not change the instructions for recording time of day observations during the survey.

A consequence of this decision, combined with the relatively low volume of heavy vehicles as compared to passenger vehicles, was the inability to interpolate time values between the limited number of time of day observations.

Absence of time values and the coincidental absence of recording gaps eliminated the need for a portion of the data preprocessing steps used with AM and PM surveys. As with the passenger vehicle surveys data transcriptions were checked for transcription consistency before conversion to .dbf format database tables.

Absence of time values in the mid-day data required a modification of the matching procedures used to detect and count through trips. By this qualification, a through trip was defined as the simple exact match between an outbound license plate and any occurrence of the same plate in the specified inbound list. This qualification was supported by visual inspection of the heavy vehicle transcriptions. Visual inspection did not reveal instances of obvious or systematic repetition of 4-character combinations which would have compromised the data.

TABLE A.08 : Mid-Day Heavy Vehicle Matches

MD Through Trucks		Outbound					
		A	B	C	D	E	F
Inbound	1		6	1	16	0	1
	2	8		4	3	6	
	3	2	1		2	1	0
	4	21	4	1		3	2
	5	4	5	9	5		0
	6	7		0	11	0	

Table A.08 shows the matrix of mid-day through trip matches. The perception that this matrix contains very low volumes should be balanced by the observation that the data represent only heavy, multi-axle vehicles – a definition excluding light and medium size trucks.

Table A.09 on the following page shows the results of the 2.5 hour mid-day heavy vehicle license plate survey in the format used previously for the AM and PM surveys. This table is organized to show through trips as components of the heavy vehicle streams entering the SR 9 corridor at inbound survey stations and also as components of the heavy vehicle streams leaving the corridor at outbound survey stations.

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Enter at 1 : 82	Volume Percent		Exit at A : 127	Volume Percent	
	Exit at B :	6 7.3%		Enter at 2 :	8 6.3%
	Exit at C :	1 1.2%		Enter at 3 :	2 1.6%
	Exit at D :	16 19.5%		Enter at 4 :	21 16.5%
	Exit at E :	0 0.0%		Enter at 5 :	4 3.1%
	Exit at F :	1 1.2%		Enter at 6 :	7 5.5%
Total Through:		24 29.3%	Total Through:		42 33.1%
To Other :		58 70.7%	From Other :		85 66.9%
Total :		82 100.0%	Total :		127 100.0%
Enter at 2 : 120	Volume Percent		Exit at B : 106	Volume Percent	
	Exit at A :	8 6.7%		Enter at 1 :	6 5.7%
	Exit at C :	4 3.3%		Enter at 3 :	1 0.9%
	Exit at D :	3 2.5%		Enter at 4 :	4 3.8%
	Exit at E :	6 5.0%		Enter at 5 :	5 4.7%
Total Through:		21 17.5%	Total Through:		16 15.1%
To Other :		99 82.5%	From Other :		90 84.9%
Total :		120 100.0%	Total :		106 100.0%
Enter at 3 : 57	Volume Percent		Exit at C : 56	Volume Percent	
	Exit at A :	2 3.5%		Enter at 1 :	1 1.8%
	Exit at B :	1 1.8%		Enter at 2 :	4 7.1%
	Exit at D :	2 3.5%		Enter at 4 :	1 1.8%
	Exit at E :	1 1.8%		Enter at 5 :	9 16.1%
	Exit at F :	0 0.0%		Enter at 6 :	0 0.0%
Total Through:		6 10.5%	Total Through:		15 26.8%
To Other :		51 89.5%	From Other :		41 73.2%
Total :		57 100.0%	Total :		56 100.0%
Enter at 4 : 73	Volume Percent		Exit at D : 124	Volume Percent	
	Exit at A :	21 28.8%		Enter at 1 :	16 12.9%
	Exit at B :	4 5.5%		Enter at 2 :	3 2.4%
	Exit at C :	1 1.4%		Enter at 3 :	2 1.6%
	Exit at E :	3 4.1%		Enter at 5 :	5 4.0%
	Exit at F :	2 2.7%		Enter at 6 :	11 8.9%
Total Through:		31 42.5%	Total Through:		37 29.8%
To Other :		42 57.5%	From Other :		87 70.2%
Total :		73 100.0%	Total :		124 100.0%
Enter at 5 : 56	Volume Percent		Exit at E : 22	Volume Percent	
	Exit at A :	4 7.1%		Enter at 1 :	0 0.0%
	Exit at B :	5 8.9%		Enter at 2 :	6 27.3%
	Exit at C :	9 16.1%		Enter at 3 :	1 4.5%
	Exit at D :	5 8.9%		Enter at 4 :	3 13.6%
	Exit at F :	0 0.0%		Enter at 6 :	0 0.0%
Total Through:		23 41.1%	Total Through:		10 45.5%
To Other :		33 58.9%	From Other :		12 54.5%
Total :		56 100.0%	Total :		22 100.0%
Enter at 6 : 43	Volume Percent		Exit at F : 19	Volume Percent	
	Exit at A :	7 16.3%		Enter at 1 :	1 5.3%
	Exit at C :	0 0.0%		Enter at 3 :	0 0.0%
	Exit at D :	11 25.6%		Enter at 4 :	2 10.5%
	Exit at E :	0 0.0%		Enter at 5 :	0 0.0%
Total Through:		18 41.9%	Total Through:		3 15.8%
To Other :		25 58.1%	From Other :		16 84.2%
Total :		43 100.0%	Total :		19 100.0%

TABLE A.09 : Through Heavy Vehicle Trip Analysis – 11:00 AM to 1:30 PM, August 22, 2001

Preliminary comparison of Table A.09 with Tables A.07 (a) and (b) reveal that the relative distribution of through trips made by heavy vehicles differs from the distribution of through trips by passenger vehicles. Notwithstanding the substantially lower numbers of heavy vehicles, percentage values are not only higher for heavy vehicles but indicate clustering, as compared to generally disperse passenger vehicles. Such clustering suggests that there may be a perception of heavy vehicle concentration along some portions of the SR 9 corridor that outweighs the actual volume of heavy vehicles.



## Evaluation of Weekday Through Trip Characteristics

The tabulation of through trips detected by the license plate survey and presented in Tables A.07 (a), A.07 (b), and A.09 reveals some evidence of directional movements associated with morning and afternoon peak traffic periods. The location and size of Greenfield relative to the Indianapolis metropolitan area contribute to logical morning and afternoon commuting patterns on I-70 and US 40.

Movements along SR 9 and through the study corridor are generally less pronounced. The alignment and extent of commercial areas along SR 9 contribute to the blurring between through trips and trips originating or ending in or near the SR 9 corridor. For example, a commuter may leave Indianapolis, enter the vicinity of Greenfield, and use SR 9 for making one or more brief stops before leaving the SR 9 corridor for home. Because of the interaction with intermediate Greenfield destinations, such a commute would not meet the definition of a through trip.

While a through trip might use the same route as the commuter, the through trip makes no intermediate stops. One of the principal objectives of the SR 9 Corridor Study is to quantify the degree of interaction between these two types of trips. Another objective will be to assess the interaction of both types of trips with strictly local travel both originating and ending in Greenfield.

At the request of INDOT, PKG and PIC expanded the findings from the 5-hour license plate survey to a 24-hour basis using the mechanical vehicle counts and vehicle classification counts conducted by INDOT concurrently with the survey. Two possible work plans were tested in the completion of this task.

One assumption was that both passenger vehicle and heavy vehicle through traffic would exhibit generally uniform patterns. If this assumption proved true, estimated through passenger vehicle trips could be deducted from total traffic volumes to show heavy vehicle through trips; however preliminary testing indicated that the data did not meet the requirements of this assumption.

The second assumption was that passenger vehicle and heavy vehicle through traffic could exhibit different overall patterns. This assumption, which proved true, required independent assessment of passenger vehicle and heavy vehicle through volumes prior to comparison with total traffic volumes.

The remainder of this chapter will describe the separate passenger vehicle and heavy vehicle analyses and the development of an overall weekday through trip estimate for the SR 9 corridor.

## ***INDOT MECHANICAL VEHICLE COUNTS AND VEHICLE CLASSIFICATION COUNTS***

INDOT mechanical vehicle counts were conducted at numerous locations in and near the SR 9 corridor in the Greenfield vicinity. Table A.10 shows the identification codes and locations for the twelve count locations corresponding to the license plate survey observation stations.

At eight of these twelve INDOT locations standard vehicle counts were recorded continuously through the entire day of the license plate survey. At four INDOT locations mechanical vehicle classification counts were conducted. Vehicle classification counters use an internal analysis of the number and timing of axles passing over the detector tubes to tabulate the number of vehicles fitting fifteen different axle profile types. Post-processing of the data enables tabulation of the various types of vehicles recorded. Such classification was vital to the analysis of the SR 9 corridor survey because it provided a reliable sampling of the number and distribution of heavy vehicles within the overall traffic streams.

TABLE A.10 : Corresponding License Plate Survey and INDOT Vehicle Count Locations

<b>License Plate Survey Station ID</b>	<b>INDOT Count ID</b>	<b>Direction and Location</b>	<b>Report Type</b>
AM/PM 1 IN	60 SB	Southbound SR 9 north of CR 500 N	<i>Classification</i>
AM/PM 2 IN	41 EB Off	I-70 Eastbound Off Ramp west of SR 9	Count
AM/PM 3 IN	70 EB	Eastbound US 40 east of CR 75 W	Count
AM/PM 4 IN	10 NB	Northbound SR 9 north of White Oak Dr	<i>Classification</i>
AM/PM 5 IN	80 WB	Westbound US 40 east of CR 300 E	Count
AM/PM 6 IN	43 WB Off	I-70 Westbound Off Ramp east of SR 9	Count
AM/PM A OUT	60 NB	Northbound SR 9 north of CR 500 N	<i>Classification</i>
AM/PM B OUT	44 WB On	I-70 Westbound On Ramp west of SR 9	Count
AM/PM C OUT	70 WB	Westbound US 40 east of CR 75 W	Count
AM/PM D OUT	10 SB	Southbound SR 9 north of White Oak Dr	<i>Classification</i>
AM/PM E OUT	80 EB	Eastbound US 40 east of CR 300 E	Count
AM/PM F OUT	42 EB On	I-70 Eastbound On Ramp east of SR 9	Count

Copies of the INDOT traffic counts are included at the end of this chapter.

Data from the four INDOT vehicle classification count locations at the northern ( 1 IN, A OUT ) and southern ( 4 IN, D OUT ) ends of the SR 9 corridor provide important information about the daily overall traffic patterns and heavy vehicle presence along SR 9. These data are presented in the following Tables A.11, A.12 and Figures A.04, A.05 ( 1 IN, A OUT ) and Tables A.13, A.14 and Figures A.06, A.07 ( 4 IN, D OUT ).

Tables include summaries for the 2.5-hour AM, PM, and Mid-Day periods when the license plate surveys were being conducted. Graphical figures are based on the data tables.

The two inbound locations show maximum peak hour traffic volumes at and near 8 AM, while the two outbound locations show maximum peak hour traffic volumes at and near 5 PM. This pattern is consistent with morning commuter traffic using SR 9 for access to I-70 and US 40 and then returning to the Greenfield vicinity in the afternoon.

In contrast to the overall traffic patterns, heavy vehicles gradually increase to a maximum peak volume in the mid-morning and remain nearly constant through the mid-afternoon before tapering off through the evening and overnight hours.

These observations suggest that there is a brief period in the morning when heavy vehicle concentrations coincide with the peak in passenger vehicle commuters. There is a brief mid-afternoon period of similar, but lower, heavy vehicle and passenger vehicle concentration. Between these two brief peaks, extending between mid-morning and early afternoon, the near constant volume of heavy vehicles becomes a higher percentage of overall traffic because there are relatively fewer automobiles.

## SR 9 Environmental Assessment / Corridor Study

TABLE A.11 : Hourly Vehicle Class Volumes at Station 1 IN

## AM/PM 1 in [ 60 SB ]

Hour Ending	Vehicle Classes		Total	Percent Pass.Veh.	Percent Trucks
	1 - 3 Pass.Veh.	4 - 15 Trucks			
1:00 AM	27	16	43	62.8%	37.2%
2:00 AM	10	11	21	47.6%	52.4%
3:00 AM	14	11	25	56.0%	44.0%
4:00 AM	21	6	27	77.8%	22.2%
5:00 AM	52	15	67	77.6%	22.4%
6:00 AM	231	14	245	94.3%	5.7%
7:00 AM	429	19	448	95.8%	4.2%
8:00 AM	508	57	565	89.9%	10.1%
9:00 AM	320	48	368	87.0%	13.0%
10:00 AM	257	50	307	83.7%	16.3%
11:00 AM	243	46	289	84.1%	15.9%
12:00 PM	237	37	274	86.5%	13.5%
1:00 PM	238	48	286	83.2%	16.8%
2:00 PM	246	56	302	81.5%	18.5%
3:00 PM	311	69	380	81.8%	18.2%
4:00 PM	317	64	381	83.2%	16.8%
5:00 PM	329	43	372	88.4%	11.6%
6:00 PM	353	39	392	90.1%	9.9%
7:00 PM	260	21	281	92.5%	7.5%
8:00 PM	198	30	228	86.8%	13.2%
9:00 PM	161	27	188	85.6%	14.4%
10:00 PM	113	14	127	89.0%	11.0%
11:00 PM	84	10	94	89.4%	10.6%
12:00 AM	30	12	42	71.4%	28.6%
24-Hour	4989	763	5752	86.7%	13.3%
6:00a- 8:30a	1097	100	1197	91.6%	8.4%
11:00a- 1:30p	598	113	711	84.1%	15.9%
3:30p - 6:00p	841	114	955	88.1%	11.9%



## SR 9 Environmental Assessment / Corridor Study

TABLE A.12 : Hourly Vehicle Class Volumes at Station A OUT

AM/PM A out [ 60 NB ]

Hour Ending	Vehicle Classes		Total	Percent Pass.Veh.	Percent Trucks
	1 - 3 Pass.Veh.	4 - 15 Trucks			
1:00 AM	40	13	53	75.5%	24.5%
2:00 AM	20	10	30	66.7%	33.3%
3:00 AM	13	6	19	68.4%	31.6%
4:00 AM	19	9	28	67.9%	32.1%
5:00 AM	19	16	35	54.3%	45.7%
6:00 AM	71	20	91	78.0%	22.0%
7:00 AM	195	36	231	84.4%	15.6%
8:00 AM	339	31	370	91.6%	8.4%
9:00 AM	239	60	299	79.9%	20.1%
10:00 AM	192	56	248	77.4%	22.6%
11:00 AM	199	60	259	76.8%	23.2%
12:00 PM	219	61	280	78.2%	21.8%
1:00 PM	279	55	334	83.5%	16.5%
2:00 PM	265	42	307	86.3%	13.7%
3:00 PM	342	56	398	85.9%	14.1%
4:00 PM	467	63	530	88.1%	11.9%
5:00 PM	529	42	571	92.6%	7.4%
6:00 PM	449	26	475	94.5%	5.5%
7:00 PM	382	18	400	95.5%	4.5%
8:00 PM	248	14	262	94.7%	5.3%
9:00 PM	222	20	242	91.7%	8.3%
10:00 PM	134	12	146	91.8%	8.2%
11:00 PM	62	14	76	81.6%	18.4%
12:00 AM	98	6	104	94.2%	5.8%
24-Hour	5042	746	5788	87.1%	12.9%
6:00a- 8:30a	654	97	751	87.1%	12.9%
11:00a- 1:30p	631	137	768	82.2%	17.8%
3:30p - 6:00p	1212	99	1311	92.4%	7.6%

FIGURE A.04 : Inbound Vehicle Class Counts and Percentages at Station 1 IN

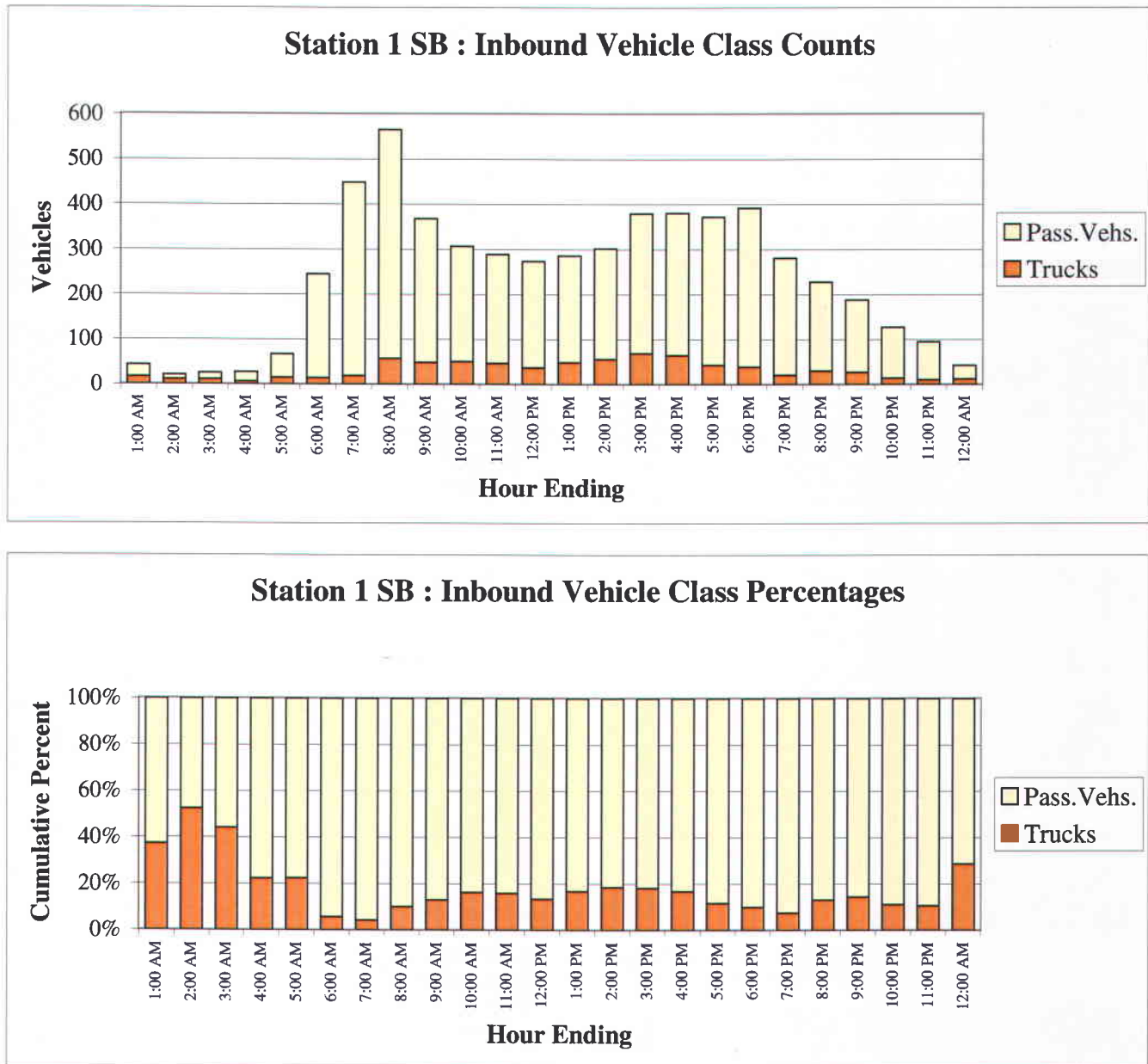
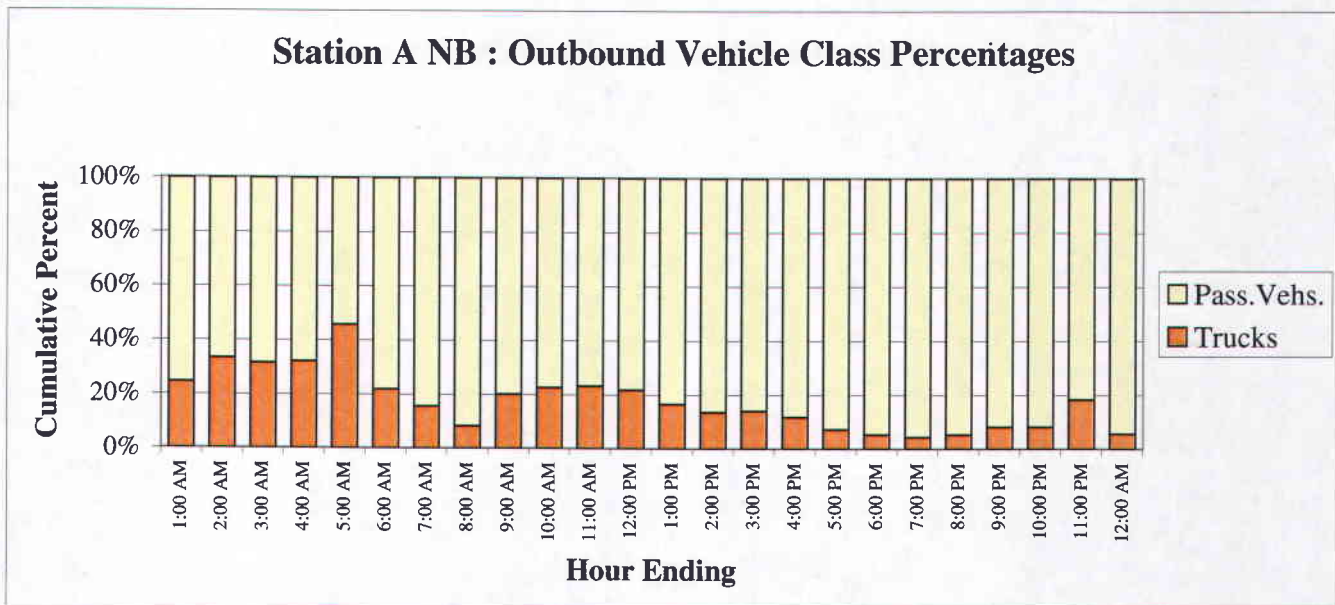
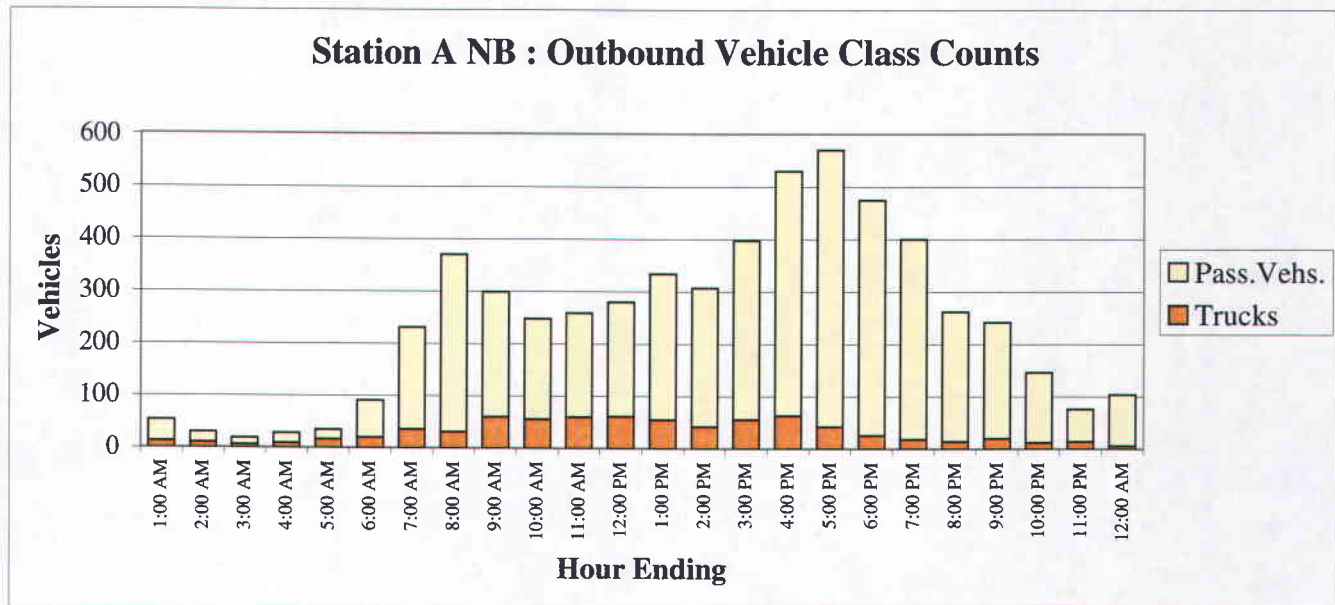


FIGURE A.05 : Outbound Vehicle Class Counts and Percentages at Station A OUT



## SR 9 Environmental Assessment / Corridor Study

TABLE A.13 : Hourly Vehicle Class Volumes at Station 4 IN

AM/PM 4 in [ 10 NB ]

Hour Ending	Vehicle Classes		Total	Percent Pass.Veh.	Percent Trucks
	1 - 3 Pass.Veh.	4 - 15 Trucks			
1:00 AM	27	14	41	65.9%	34.1%
2:00 AM	12	10	22	54.5%	45.5%
3:00 AM	14	8	22	63.6%	36.4%
4:00 AM	13	11	24	54.2%	45.8%
5:00 AM	41	15	56	73.2%	26.8%
6:00 AM	146	24	170	85.9%	14.1%
7:00 AM	295	27	322	91.6%	8.4%
8:00 AM	388	47	435	89.2%	10.8%
9:00 AM	252	40	292	86.3%	13.7%
10:00 AM	230	44	274	83.9%	16.1%
11:00 AM	209	43	252	82.9%	17.1%
12:00 PM	206	46	252	81.7%	18.3%
1:00 PM	237	37	274	86.5%	13.5%
2:00 PM	245	35	280	87.5%	12.5%
3:00 PM	250	56	306	81.7%	18.3%
4:00 PM	275	33	308	89.3%	10.7%
5:00 PM	289	34	323	89.5%	10.5%
6:00 PM	323	27	350	92.3%	7.7%
7:00 PM	246	17	263	93.5%	6.5%
8:00 PM	183	12	195	93.8%	6.2%
9:00 PM	119	22	141	84.4%	15.6%
10:00 PM	94	14	108	87.0%	13.0%
11:00 PM	65	10	75	86.7%	13.3%
12:00 AM	43	6	49	87.8%	12.2%
24-Hour	4202	632	4834	86.9%	13.1%
6:00a- 8:30a	809	94	903	89.6%	10.4%
11:00a- 1:30p	566	100	666	85.0%	15.0%
3:30p - 6:00p	750	77	827	90.7%	9.3%





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TABLE A.14 : Hourly Vehicle Class Volumes at Station D OUT

## AM/PM D out [ 10 SB ]

Hour Ending	Vehicle Classes		Total	Percent Pass.Veh.	Percent Trucks
	1 - 3 Pass.Veh.	4 - 15 Trucks			
1:00 AM	23	18	41	56.1%	43.9%
2:00 AM	16	11	27	59.3%	40.7%
3:00 AM	11	11	22	50.0%	50.0%
4:00 AM	16	13	29	55.2%	44.8%
5:00 AM	33	13	46	71.7%	28.3%
6:00 AM	101	22	123	82.1%	17.9%
7:00 AM	199	31	230	86.5%	13.5%
8:00 AM	201	49	250	80.4%	19.6%
9:00 AM	199	50	249	79.9%	20.1%
10:00 AM	208	70	278	74.8%	25.2%
11:00 AM	207	59	266	77.8%	22.2%
12:00 PM	213	48	261	81.6%	18.4%
1:00 PM	219	47	266	82.3%	17.7%
2:00 PM	240	54	294	81.6%	18.4%
3:00 PM	260	46	306	85.0%	15.0%
4:00 PM	347	42	389	89.2%	10.8%
5:00 PM	344	37	381	90.3%	9.7%
6:00 PM	374	32	406	92.1%	7.9%
7:00 PM	311	18	329	94.5%	5.5%
8:00 PM	217	30	247	87.9%	12.1%
9:00 PM	172	31	203	84.7%	15.3%
10:00 PM	124	23	147	84.4%	15.6%
11:00 PM	72	13	85	84.7%	15.3%
12:00 AM	52	11	63	82.5%	17.5%
24-Hour	4159	779	4938	84.2%	15.8%
6:00a- 8:30a	500	105	605	82.6%	17.4%
11:00a- 1:30p	552	122	674	81.9%	18.1%
3:30p - 6:00p	892	90	982	90.8%	9.2%

FIGURE A.06 : Inbound Vehicle Class Counts and Percentages at Station 4 IN

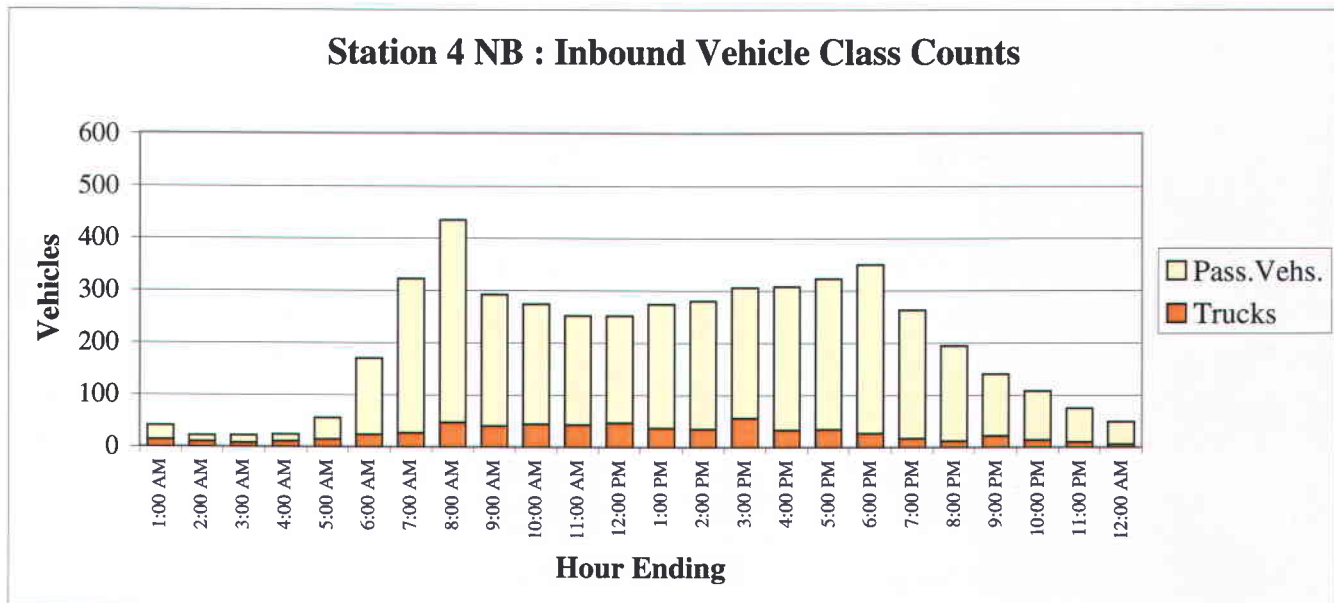
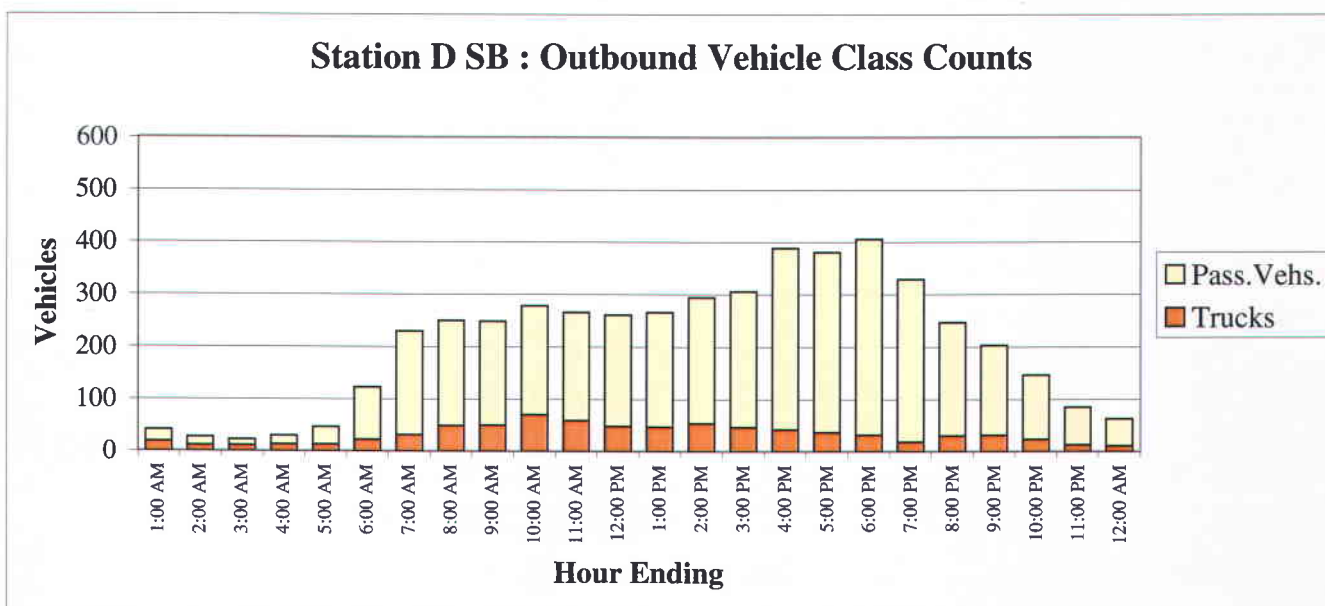
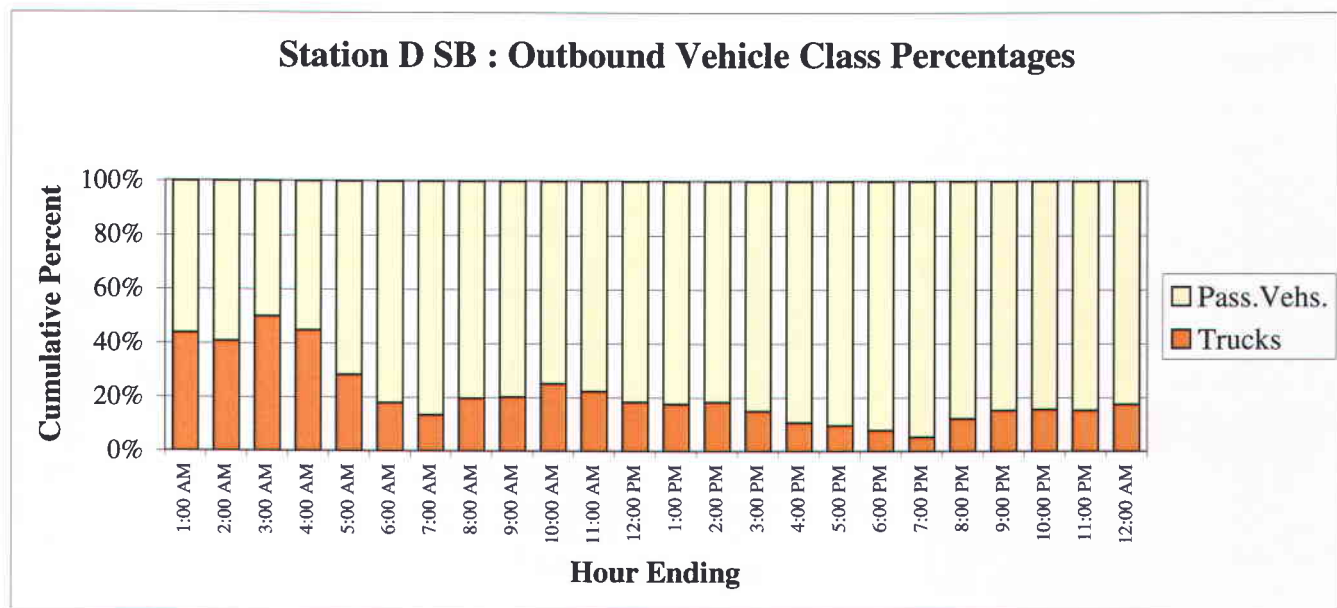


FIGURE A.07 : Outbound Vehicle Class Counts and Percentages at Station D OUT



As stated previously in this chapter, the pattern of origins and destinations for heavy vehicle through trips using the SR 9 corridor is different from the patterns for passenger vehicles. This fact, combined with the time-of-day volume differences between heavy vehicles and passenger vehicles, was the basis for the decision to disaggregate vehicle types in the subsequent task of applying peak hour through trip characteristics to an expanded 24-hour estimate.

In addition to evaluation of the INDOT classification counts themselves, peak hour data were entered into a standard INDOT worksheet used to determine axle adjustment factors and heavy vehicle percentages. When calculated using vehicle classification counts, axle correction factors provide a standardized method for correcting mechanical vehicle overcounts attributed to the excess number of axles on heavy vehicles.

In the tables that follow, the sum of the column labeled *Actual Volume* can be multiplied by the calculated *Axle Correction Factor (ACF)* to yield an adjusted total volume. The calculated % *Heavy Vehicles (%HV)* can then be applied to the adjusted total volume to determine the adjusted total number of heavy vehicles and passenger vehicles.





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Table A.14 shows Axle Correction Factor calculations for a full 24-hours at the four stations where vehicle classification count data were available. Tables A.15, A.16, and A.17 show the calculations for the 2.5-hour AM, PM, and Mid-Day survey periods, respectively.

TABLE A.14 : Axle Correction Factor Worksheet – 24-Hour Vehicle Data

**Location: SR 9 at CR 500 N [Inbound]**  
Sta.1 IN southbound

Class	Actual Volume	Excess Vehicles	Excess Axles	Act. Volume + Excess Axles
1	3	0	0	3
2	830	0	0	830
3	264	0	0	264
4	0	0	0	0
5	18	0	0	18
6	1	0.5	0.5	1.5
7	5	1	5	10
8	12	1	12	24
9	40	1.5	60	100
10	0	2	0	0
11	2	1.5	3	5
12	24	2	48	72
13	1	2.5	2.5	3.5
<b>Totals:</b>	<b>1,200</b>			<b>1,331</b>

Axle Correction Factor =  
0.902

%HV = 8.6%

**Location: SR 9 at CR 500 N [Outbound]**  
Sta.A OUT northbound

Class	Actual Volume	Excess Vehicles	Excess Axles	Act. Volume + Excess Axles
1	5	0	0	5
2	473	0	0	473
3	176	0	0	176
4	0	0	0	0
5	26	0	0	26
6	18	0.5	9	27
7	0	1	0	0
8	5	1	5	10
9	40	1.5	60	100
10	1	2	2	3
11	0	1.5	0	0
12	8	2	16	24
13	0	2.5	0	0
<b>Totals:</b>	<b>752</b>			<b>844</b>

Axle Correction Factor =  
0.891

%HV = 13.0%

**Location: SR 9 at White Oak [Inbound]**  
Sta.4 IN northbound

Class	Actual Volume	Excess Vehicles	Excess Axles	Act. Volume + Excess Axles
1	2	0	0	2
2	581	0	0	581
3	226	0	0	226
4	1	0	0	1
5	14	0	0	14
6	16	0.5	8	24
7	6	1	6	12
8	6	1	6	12
9	51	1.5	76.5	127.5
10	1	2	2	3
11	0	1.5	0	0
12	1	2	2	3
13	0	2.5	0	0
<b>Totals:</b>	<b>905</b>			<b>1,006</b>

Axle Correction Factor =  
0.900

%HV = 10.6%

**Location: SR 9 at White Oak [Outbound]**  
Sta.D OUT southbound

Class	Actual Volume	Excess Vehicles	Excess Axles	Act. Volume + Excess Axles
1	4	0	0	4
2	352	0	0	352
3	144	0	0	144
4	0	0	0	0
5	15	0	0	15
6	13	0.5	6.5	19.5
7	4	1	4	8
8	12	1	12	24
9	61	1.5	91.5	152.5
10	0	2	0	0
11	0	1.5	0	0
12	1	2	2	3
13	0	2.5	0	0
<b>Totals:</b>	<b>606</b>			<b>722</b>

Axle Correction Factor =  
0.839

%HV = 17.5%



TABLE A.15 : Axle Correction Factor Worksheet – 2.5-Hour AM Vehicle Data

**Location: SR 9 at CR 500 N [Inbound]**  
Sta.1 IN southbound

Class	Actual Volume	Excess Vehicles	Excess Axles	Act. Volume + Excess Axles
1	33	0	0	33
2	3744	0	0	3744
3	1212	0	0	1212
4	3	0	0	3
5	84	0	0	84
6	36	0.5	18	54
7	72	1	72	144
8	66	1	66	132
9	391	1.5	586.5	977.5
10	5	2	10	15
11	4	1.5	6	10
12	98	2	196	294
13	4	2.5	10	14
<b>Totals:</b>	5,752			6,717

Axle Correction Factor =  
0.856

%HV = 13.3%

**Location: SR 9 at CR 500 N [Outbound]**  
Sta.A OUT northbound

Class	Actual Volume	Excess Vehicles	Excess Axles	Act. Volume + Excess Axles
1	47	0	0	47
2	3820	0	0	3820
3	1175	0	0	1175
4	2	0	0	2
5	97	0	0	97
6	163	0.5	81.5	244.5
7	4	1	4	8
8	44	1	44	88
9	386	1.5	579	965
10	6	2	12	18
11	4	1.5	6	10
12	39	2	78	117
13	1	2.5	2.5	3.5
<b>Totals:</b>	5,788			6,595

Axle Correction Factor =  
0.878

%HV = 12.9%

**Location: SR 9 at White Oak [Inbound]**  
Sta.4 IN northbound

Class	Actual Volume	Excess Vehicles	Excess Axles	Act. Volume + Excess Axles
1	26	0	0	26
2	3074	0	0	3074
3	1102	0	0	1102
4	3	0	0	3
5	63	0	0	63
6	62	0.5	31	93
7	25	1	25	50
8	47	1	47	94
9	420	1.5	630	1050
10	5	2	10	15
11	1	1.5	1.5	2.5
12	5	2	10	15
13	1	2.5	2.5	3.5
<b>Totals:</b>	4,834			5,591

Axle Correction Factor =  
0.865

%HV = 13.1%

**Location: SR 9 at White Oak [Outbound]**  
Sta.D OUT southbound

Class	Actual Volume	Excess Vehicles	Excess Axles	Act. Volume + Excess Axles
1	18	0	0	18
2	3021	0	0	3021
3	1120	0	0	1120
4	5	0	0	5
5	76	0	0	76
6	77	0.5	38.5	115.5
7	16	1	16	32
8	82	1	82	164
9	504	1.5	756	1260
10	7	2	14	21
11	0	1.5	0	0
12	9	2	18	27
13	3	2.5	7.5	10.5
<b>Totals:</b>	4,938			5,870

Axle Correction Factor =  
0.841

%HV = 15.8%

TABLE A.16 : Axle Correction Factor Worksheet – 2.5-Hour PM Vehicle Data

## Location: SR 9 at CR 500 N [Inbound]

Sta. 1 IN southbound

Class	Actual Volume	Excess Vehicles	Excess Axles	Act. Volume + Excess Axles
1	9	0	0	9
2	613	0	0	613
3	219	0	0	219
4	1	0	0	1
5	15	0	0	15
6	9	0.5	4.5	13.5
7	10	1	10	20
8	8	1	8	16
9	60	1.5	90	150
10	0	2	0	0
11	0	1.5	0	0
12	10	2	20	30
13	1	2.5	2.5	3.5
Totals:	955			1,090

Axle Correction Factor =  
0.876

%HV = 11.9%

## Location: SR 9 at CR 500 N [Outbound]

Sta. A OUT northbound

Class	Actual Volume	Excess Vehicles	Excess Axles	Act. Volume + Excess Axles
1	14	0	0	14
2	949	0	0	949
3	249	0	0	249
4	0	0	0	0
5	17	0	0	17
6	14	0.5	7	21
7	2	1	2	4
8	7	1	7	14
9	54	1.5	81	135
10	1	2	2	3
11	1	1.5	1.5	2.5
12	5	2	10	15
13	0	2.5	0	0
Totals:	1,313			1,424

Axle Correction Factor =  
0.922

%HV = 7.7%

## Location: SR 9 at White Oak [Inbound]

Sta. 4 IN northbound

Class	Actual Volume	Excess Vehicles	Excess Axles	Act. Volume + Excess Axles
1	8	0	0	8
2	558	0	0	558
3	185	0	0	185
4	1	0	0	1
5	9	0	0	9
6	7	0.5	3.5	10.5
7	3	1	3	6
8	5	1	5	10
9	54	1.5	81	135
10	1	2	2	3
11	0	1.5	0	0
12	0	2	0	0
13	0	2.5	0	0
Totals:	831			926

Axle Correction Factor =  
0.898

%HV = 9.6%

## Location: SR 9 at White Oak [Outbound]

Sta. D OUT southbound

Class	Actual Volume	Excess Vehicles	Excess Axles	Act. Volume + Excess Axles
1	5	0	0	5
2	645	0	0	645
3	242	0	0	242
4	0	0	0	0
5	7	0	0	7
6	10	0.5	5	15
7	3	1	3	6
8	11	1	11	22
9	58	1.5	87	145
10	1	2	2	3
11	0	1.5	0	0
12	1	2	2	3
13	1	2.5	2.5	3.5
Totals:	984			1,097

Axle Correction Factor =  
0.897

%HV = 9.3%

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TABLE A.17 : Axle Correction Factor Worksheet – 2.5-Hour Mid-Day Vehicle Data

## Location: SR 9 at CR 500 N [Inbound]

Sta.1 IN southbound

Class	Actual Volume	Excess Vehicles	Excess Axles	Act. Volume + Excess Axles
1	0	0	0	0
2	457	0	0	457
3	142	0	0	142
4	0	0	0	0
5	9	0	0	9
6	5	0.5	2.5	7.5
7	24	1	24	48
8	8	1	8	16
9	60	1.5	90	150
10	2	2	4	6
11	0	1.5	0	0
12	7	2	14	21
13	0	2.5	0	0
Totals:	714			857

Axle Correction Factor =  
0.834

%HV = 16.1%

## Location: SR 9 at CR 500 N [Outbound]

Sta.A OUT northbound

Class	Actual Volume	Excess Vehicles	Excess Axles	Act. Volume + Excess Axles
1	6	0	0	6
2	476	0	0	476
3	149	0	0	149
4	1	0	0	1
5	14	0	0	14
6	48	0.5	24	72
7	1	1	1	2
8	9	1	9	18
9	58	1.5	87	145
10	1	2	2	3
11	0	1.5	0	0
12	7	2	14	21
13	0	2.5	0	0
Totals:	770			907

Axle Correction Factor =  
0.849

%HV = 18.1%

## Location: SR 9 at White Oak [Inbound]

Sta.4 IN northbound

Class	Actual Volume	Excess Vehicles	Excess Axles	Act. Volume + Excess Axles
1	3	0	0	3
2	401	0	0	401
3	162	0	0	162
4	0	0	0	0
5	6	0	0	6
6	13	0.5	6.5	19.5
7	5	1	5	10
8	11	1	11	22
9	63	1.5	94.5	157.5
10	1	2	2	3
11	0	1.5	0	0
12	3	2	6	9
13	0	2.5	0	0
Totals:	668			793

Axle Correction Factor =  
0.842

%HV = 15.3%

## Location: SR 9 at White Oak [Outbound]

Sta.D OUT southbound

Class	Actual Volume	Excess Vehicles	Excess Axles	Act. Volume + Excess Axles
1	0	0	0	0
2	387	0	0	387
3	165	0	0	165
4	0	0	0	0
5	18	0	0	18
6	12	0.5	6	18
7	1	1	1	2
8	11	1	11	22
9	77	1.5	115.5	192.5
10	1	2	2	3
11	0	1.5	0	0
12	3	2	6	9
13	1	2.5	2.5	3.5
Totals:	676			820

Axle Correction Factor =  
0.824

%HV = 18.3%



This series of worksheets reveals an important validation of previous comments regarding the time of day variations in heavy vehicle percentages of the SR 9 traffic volumes. Axle correction factors average roughly 0.9 in the AM and PM periods when passenger vehicle percentages of the traffic streams are highest. During the mid-day, when heavy vehicle percentages are relatively higher, axle correction factors are slightly above 0.8 as a consequence of the number of vehicles with more than two axles. On the 24-hour worksheet, an average axle correction factors in the mid-range between 0.8 and 0.9 confirm that, on a daily basis, passenger vehicles comprise most of the SR 9 traffic stream.

Moreover, there is a logical consistency between the worksheets. This characteristic supports the use of these axle correction factors, based on available vehicle classification data, to adjust the raw mechanical vehicle counts at other stations. In the absence of vehicle classification counts at all stations along the SR 9 corridor, the validity of such adjustments is an important element in the expansion of peak period through traffic distributions to estimated daily distributions of through trips.

## Synthesis and Validation of License Plate Survey and Vehicle Count Data

Previous sections of this chapter have described the methods for collecting and assembling the data needed to develop daily through trip estimates for the SR 9 corridor. License plate data consisted of a temporal sampling [5 hours out of 24 for autos; 2.5 hours out of 24 for trucks], while vehicle classification counts were conducted continuously at only 4 of 12 vehicle count locations. This section will discuss the methodology and results of the effort to assemble the data in a unified form for direct application in the expansion of the sampled data into a 24-hour format.

As noted in earlier sections of this chapter [See Evaluation of Weekday Through Trip Characteristics], the work program selected by PKG and PIC was based upon a determination that separate estimates of passenger vehicles and heavy vehicles in relationship to overall traffic volumes would have to be developed. The following discussion will explain how these estimates were created and will identify both similarities and differences between the procedures used for the two vehicle types.

### Passenger Vehicle Estimation Procedure

Previously described Table A.06 showed the sums of adjusted passenger vehicles and total vehicles during the AM and PM license plate survey periods. With slight rounding differences attributed to matrix balancing operations, these volumes are repeated in Table A.18 along with the nominal INDOT mechanical counts for the survey periods.



Estimated (600-830) AM and (330-600) PM vehicle counts were adjusted by multiplying the [nominal value] x [Axle Correction Factor] x [100% - % Heavy Vehicles] for autos, and [nominal value] x [Axle Correction Factor] x [% Heavy Vehicles] for trucks. *Calculated Autos* and *Calculated Trucks* are added to give the *Adjusted Total Vehicle Count*. An axle correction factor of 0.891, used for the eight stations lacking vehicle classification count data, is the average ACF calculated from the four stations with classification counts.

The *Survey/Auto Ratio* equals *LPS Vehicles* divided by *Calculated Autos*; *LPS Through* vehicles divided by this ratio yields *Adjusted Through Autos*. Use of this ratio indexes the number of auto through trips directly to the refined estimates of total auto traffic.

*Adjusted Through Percent* equals *Adjusted Through Autos* divided by *Calculated Autos*. This value may be compared to the preliminary time- and volume-expanded through vehicle percentages appearing in Table A.07 (a) and (b).

The ratio [AVC]/[MVC] is the *Adjusted Total Vehicle Count* divided by the *Estimated Mechanical Vehicle Count* and is a comparable to the given *Axle Correction Factor*.

*Calculated Percent Autos* or *Percent Trucks* equal *Calculated Autos* or *Calculated Percent Trucks* divided by the *Adjusted Total Vehicle Count* and may be compared to the given *INDOT Percent Trucks*. *INDOT Percent Trucks* were calculated and checked against INDOT data at the four stations with vehicle classification counts; values at the remaining eight stations are estimates developed by review of the classification counts and field observation.

As noted in the two preceding paragraphs, Table A.18 includes comparison calculations used to monitor and test the validity of the procedures described.



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TABLE A.18 : Final Estimate of AM and PM Autos, Trucks, Total Vehicles and Through Autos

Station ID	LPS Vehicles	LPS Through	[MVC] * Estimated Mech. Veh 600-830		Adjusted Total Veh Count	Survey / Auto Ratio	Adjusted Through Autos	Adjusted Through Percent	Ratio [AVC] [MVC]	Axle Corr.Fac. [ACF]	INDOT Percent Trucks	Calculated		Calculated Percent Trucks	Autos
												Trucks	Autos		
AM 1 in	971	167	1197	1080	0.984	170	17.2%	0.902	0.902	0.902	8.6%	93	987	8.6%	91.4%
AM 2 in	697	29	1041	927	0.854	34	4.2%	0.890	0.891	0.891	12.0%	111	816	12.0%	88.0%
AM 3 in	1151	47	1198	1067	1.135	41	4.0%	0.891	0.891	0.891	5.0%	53	1014	5.0%	95.0%
AM 4 in	874	91	903	813	1.202	76	10.5%	0.900	0.900	0.900	10.6%	86	727	10.6%	89.4%
AM 5 in	943	93	1191	1061	0.955	97	9.8%	0.891	0.891	0.891	7.0%	74	987	7.0%	93.0%
AM 6 in	366	15	528	470	0.884	17	4.1%	0.890	0.891	0.891	12.0%	56	414	11.9%	88.1%
AM A out	713	84	751	669	1.225	69	11.9%	0.891	0.891	0.891	13.0%	87	582	13.0%	87.0%
AM B out	2332	198	2837	2527	1.049	189	8.5%	0.891	0.891	0.891	12.0%	303	2224	12.0%	88.0%
AM C out	1081	74	1204	1073	1.083	68	6.8%	0.891	0.891	0.891	7.0%	75	998	7.0%	93.0%
AM D out	565	25	605	508	1.348	19	4.5%	0.840	0.839	0.839	17.5%	89	419	17.5%	82.5%
AM E out	456	39	545	485	0.989	39	8.5%	0.890	0.891	0.891	5.0%	24	461	4.9%	95.1%
AM F out	334	22	466	415	0.915	24	6.6%	0.891	0.891	0.891	12.0%	50	365	12.0%	88.0%
PM 1 in	769	116	955	837	1.043	111	15.1%	0.876	0.876	0.876	11.9%	100	737	11.9%	88.1%
PM 2 in	2474	318	2505	2232	1.205	264	12.9%	0.891	0.891	0.891	8.0%	179	2053	8.0%	92.0%
PM 3 in	1788	148	1986	1769	1.064	139	8.3%	0.891	0.891	0.891	5.0%	88	1681	5.0%	95.0%
PM 4 in	819	109	827	742	1.221	89	13.3%	0.897	0.898	0.898	9.6%	71	671	9.6%	90.4%
PM 5 in	737	80	954	851	0.932	86	10.9%	0.892	0.891	0.891	7.0%	60	791	7.1%	92.9%
PM 6 in	378	31	511	456	0.943	33	8.2%	0.892	0.891	0.891	12.0%	55	401	12.1%	87.9%
PM A out	1323	367	1311	1209	1.185	310	27.8%	0.922	0.922	0.922	7.7%	93	1116	7.7%	92.3%
PM B out	1083	92	1357	1209	1.018	90	8.5%	0.891	0.891	0.891	12.0%	145	1064	12.0%	88.0%
PM C out	1576	96	1610	1435	1.156	83	6.1%	0.891	0.891	0.891	5.0%	72	1363	5.0%	95.0%
PM D out	913	80	982	881	1.143	70	8.8%	0.897	0.897	0.897	9.3%	82	799	9.3%	90.7%
PM E out	1068	144	1203	1072	1.071	134	13.4%	0.891	0.891	0.891	7.0%	75	997	7.0%	93.0%
PM F out	469	23	634	565	0.944	24	4.8%	0.891	0.891	0.891	12.0%	68	497	12.0%	88.0%



### Heavy Vehicle Estimation Procedure

Estimates of heavy vehicle totals and through trips were refined using a similar spreadsheet method described for passenger vehicles. Table A.19 is adapted from the working spreadsheet in which the notation "truck" is substituted for "heavy vehicle" for the conservation of space. Any references to "trucks" are equally representative of trucks or buses.

It is important to re-emphasize that the mid-day license plate survey collected data only on heavy vehicles, not on passenger vehicles. Mechanical volume and vehicle classification counts were based on all vehicles. Therefore, consistency of methodology required recalculation of heavy vehicles and passenger vehicles to derive adjusted total vehicles and the relationship of that value to through heavy vehicle trips.

Previously described Table A.09 showed the sums of adjusted heavy vehicle through trips and total heavy vehicles during the mid-day license plate survey period. These volumes are repeated in Table A.19 along with the nominal INDOT mechanical count for that survey period.

Estimated (1100AM-130PM) total vehicle counts were adjusted by multiplying the *[nominal value] x [Axle Correction Factor] x [100% - % Heavy Vehicles]* for autos, and *[nominal value] x [Axle Correction Factor] x [% Heavy Vehicles]* for trucks. *Calculated Autos* and *Calculated Trucks* are added to give the *Adjusted Total Vehicle Count*. Estimated axle correction factors of 0.838 (inbound stations) and 0.837 (outbound stations), used for the eight stations lacking vehicle classification count data, are averages for directional ACFs calculated from the four stations with classification counts.

The *Survey/Truck Ratio* equals *LPS Total Trucks* divided by *Calculated Trucks*; *LPS Through* vehicles divided by this ratio yields *Adjusted Through Trucks*. Recall that the mid-day survey counted only heavy vehicles. The Survey/Truck ratio is used to index the number of through truck trips directly to the refined estimates of total vehicles, as opposed to total heavy vehicles.

*Adjusted Through Percent* equals *Adjusted Through Trucks* divided by *Calculated Trucks*. This value may be compared to the preliminary volume-expanded through vehicle percentages appearing in Table A.09.

*Calculated Percent Autos* or *Percent Trucks* equal *Calculated Autos* or *Calculated Percent Trucks* divided by the *Adjusted Total Vehicle Count* and may be compared to the given *INDOT Percent Trucks*. *INDOT Percent Trucks* were calculated and checked against INDOT data at the four stations with vehicle classification counts. In contrast to the passenger vehicle procedure, *INDOT Percent Trucks* values at the remaining eight stations were trial-and-error estimates adjusted to bring respective *Survey/Truck Ratios* into the range of values calculated for the stations with INDOT vehicle classification data.



As noted in the two preceding paragraphs, Table A.19 includes comparison calculations used to monitor and test the validity of the procedures described.



TABLE A.19 : Final Estimate of Mid-Day Autos, Trucks, Total Vehicles and Through Trucks

Station ID	LPS Total Trucks	LPS Through Trucks	Estimated Mech.Veh 600-830	Adjusted Total.Veh Count	Survey / Truck Ratio	Adjusted Through Trucks	Adjusted Through Percent	Axle Corr.Fac. [ACF]	INDOT Percent Trucks	Calculated	
										Trucks	Autos
MD 1 in	82	24	711	593	0.863	28	29.5%	0.834	16.1%	95	498
MD 2 in	120	21	1126	944	0.845	25	17.6%	0.838	15.0%	142	802
MD 3 in	57	6	1215	1018	0.864	7	10.6%	0.838	6.5%	66	952
MD 4 in	73	31	666	561	0.849	37	43.0%	0.842	15.3%	86	475
MD 5 in	56	23	951	797	0.824	28	41.2%	0.838	8.5%	68	729
MD 6 in	43	18	447	374	0.827	22	42.3%	0.838	14.0%	52	322
MD A out	127	42	768	652	1.076	39	33.1%	0.849	18.1%	118	534
MD B out	106	16	1121	938	1.029	16	15.5%	0.837	11.0%	103	835
MD C out	56	15	1216	1018	1.098	14	27.5%	0.837	5.0%	51	967
MD D out	124	37	674	556	1.216	30	29.4%	0.824	18.3%	102	454
MD E out	22	10	737	617	1.000	10	45.5%	0.837	3.5%	22	595
MD F out	19	3	486	406	1.056	3	16.7%	0.837	4.5%	18	388
										Trucks	Autos
										Percent	Trucks
										Percent	Autos

Application of Calculated Sample Parameters to 24-Hour Data

Based on the evaluation of Tables A.18 and A.19, relationships between vehicle classification counts and axle correction factors appear to be reasonably consistent within the sampled license plate survey and classification data despite differences attributable to vehicle types. Determining this logical consistency allows uniform application of the calculated parameters to the 24-hour vehicle count data.

Table A.14 described earlier in this chapter shows the calculated axle correction factors and percent heavy vehicles using the full 24-hour vehicle classification counts at four of the twelve SR 9 corridor stations. Results from this worksheet were re-evaluated with respect to the cumulative information assembled during the testing of the sampled data. The final procedural decision organizes the 24-hour vehicle counts by inbound and outbound directions separately for passenger vehicles and heavy vehicles.

Tables A.20 through A.23 display the worksheets used to calculate directional 24-hour passenger and heavy vehicle volumes for all stations on the perimeter of the SR 9 corridor. The left half of each worksheet shows the hourly traffic volumes for six respective inbound or outbound stations. The right half displays the calculated hourly passenger vehicle or heavy vehicle values resulting from the axle correction and vehicle type percentage factors used for each station. 24-hour total vehicles are the sums of hourly calculations.

In the spreadsheet procedure, passenger vehicle percentages (see Tables A.20, A.21) were selected first. Counterpart heavy vehicle percentages were constrained by the following logical requirement :

$$\begin{aligned} X\% \text{ autos} + Y\% \text{ trucks} &= 100\% \text{ total vehicles;} \\ \text{If } X\% \text{ autos are determined first, then } Y\% \text{ trucks must} &= (100\% - X\%) \end{aligned}$$

Tables A.22 and A.23 show independently calculated, or unconstrained, heavy vehicle axle correction and heavy vehicle percentage factors for the purpose of comparison to the constrained factors used. Differences between these factors support the argument that the through trip estimation methodology has not overestimated the percentage of heavy vehicles.

In each of these four tables adjusted 24-hour total volumes by specified vehicle type provide the station control totals subsequently used to develop the 24-hour through trip matrices.

Table A.20 shows adjusted 24-hour *inbound* volumes for passenger vehicles (stations 1–6);

Table A.21 shows adjusted 24-hour *outbound* volumes for passenger vehicles (stations A–F);

Table A.22 shows adjusted 24-hour *inbound* volumes for heavy vehicles (stations 1–6);

Table A.23 shows adjusted 24-hour *outbound* volumes for heavy vehicles (stations A–F).



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Table A.20 : Calculation of 24-Hour Inbound Passenger Vehicles

Unadjusted INDOT Mechanical Vehicle Counts										Axle Adjusted INDOT Mechanical Counts : Inbound Passenger Vehicles									
Hour Ending	LPS 1 in 60 SB	LPS 2 in 41 EB off	LPS 3 in 70 EB	LPS 4 in 10 NB	LPS 5 in 80 WB	LPS 6 in 43 WB off	Total IN	Pct.Pass. Veh. Hour Ending		0.856 LPS 1 in 60 SB	0.861 LPS 2 in 41 EB off	0.861 LPS 3 in 70 EB	0.865 LPS 4 in 10 NB	0.861 LPS 5 in 80 WB	0.861 LPS 6 in 43 WB off	86.8%	86.8%	86.8%	Total IN
1:00 AM	43	87	42	41	67	40	320	1:00 AM		32	65	31	31	50	30				239
2:00 AM	21	72	35	22	15	15	180	2:00 AM		16	54	26	17	11	11				135
3:00 AM	25	40	14	22	23	27	151	3:00 AM		19	30	10	17	17	20				113
4:00 AM	27	58	23	24	38	19	189	4:00 AM		20	43	17	18	28	14				140
5:00 AM	67	98	28	56	64	85	398	5:00 AM		50	73	21	42	48	64				298
6:00 AM	245	195	123	170	206	173	1112	6:00 AM		182	146	92	128	154	129				831
7:00 AM	448	357	397	322	466	235	2225	7:00 AM		332	267	297	242	348	176				1662
8:00 AM	565	460	570	435	512	216	2758	8:00 AM		419	344	426	327	383	161				2060
9:00 AM	368	448	462	292	425	154	2149	9:00 AM		273	335	345	219	318	115				1605
10:00 AM	307	397	405	274	346	173	1902	10:00 AM		228	297	303	206	259	129				1422
11:00 AM	289	366	415	252	353	176	1851	11:00 AM		214	274	310	189	264	132				1383
12:00 PM	274	434	422	252	365	186	1933	12:00 PM		203	324	315	189	273	139				1443
1:00 PM	286	452	508	274	386	159	2065	1:00 PM		212	338	380	206	288	119				1543
2:00 PM	302	479	569	280	399	203	2232	2:00 PM		224	358	425	210	298	152				1667
3:00 PM	380	611	593	306	405	201	2496	3:00 PM		282	457	443	230	303	150				1865
4:00 PM	381	809	692	308	390	187	2767	4:00 PM		283	605	517	232	291	140				2068
5:00 PM	372	1027	799	323	432	193	3146	5:00 PM		276	768	597	243	323	144				2351
6:00 PM	392	1073	841	350	327	224	3207	6:00 PM		291	802	629	263	244	167				2396
7:00 PM	281	694	610	263	352	122	2322	7:00 PM		209	519	456	198	263	91				1736
8:00 PM	228	449	393	195	254	110	1629	8:00 PM		169	336	294	147	190	82				1218
9:00 PM	188	331	358	141	182	129	1329	9:00 PM		140	247	268	106	136	96				993
10:00 PM	127	315	258	108	158	91	1057	10:00 PM		94	235	193	81	118	68				789
11:00 PM	94	192	108	75	87	96	652	11:00 PM		70	143	81	56	65	72				487
12:00 AM	42	133	84	49	40	30	378	12:00 AM		31	99	63	37	30	22				282
24-Hour	5752	9577	8749	4834	6292	3244	38448	24-Hour		4269	7159	6539	3634	4702	2423				28726
6:00a- 8:30a	1197	1041	1198	903	1191	528	6058	6:00a- 8:30a		888	779	896	679	890	395				4525
3:30p - 6:00p	955	2505	1986	827	954	511	7737	3:30p - 6:00p		709	1873	1485	622	713	381				5781
LPS-5Hr	2152	3546	3184	1730	2145	1039	13795	LPS-5Hr		1597	2652	2381	1301	1603	776				10306
% of 24-Hr	37.4%	37.0%	36.4%	35.8%	34.1%	32.0%	35.9%	% of 24-Hr		37.4%	37.0%	36.4%	35.8%	34.1%	32.0%				35.9%
Remain-19Hr	3600	6031	5565	3104	4147	2205	24653	Remain-19Hr		2672	4507	4158	2333	3099	1647				18420
% of 24-Hr	62.6%	63.0%	63.6%	64.2%	65.9%	68.0%	64.1%	% of 24-Hr		62.6%	63.0%	63.6%	64.2%	65.9%	68.0%				64.1%



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Table A.21 : Calculation of 24-Hour Outbound Passenger Vehicles

Unadjusted INDOT Mechanical Vehicle Counts										Axle Adjusted INDOT Mechanical Counts : Outbound Passenger Vehicles																
Hour Ending	LPS A out 60 NB		LPS B out 44 WB on		LPS C out 70 WB		LPS D out 10 SB		LPS E out 80 EB		LPS F out 42 EB on		Total OUT	Pet.Pass. Veh. Hour Ending	87.1 % LPS A out 60 NB		85.6 % LPS B out 44 WB on		85.6 % LPS C out 70 WB		84.2 % LPS D out 10 SB		85.6 % LPS E out 80 EB		85.6 % LPS F out 42 EB on	Total OUT
1:00 AM	53	94	28	41	37	64	317							1:00 AM	41	69	21	29	27	47	234					
2:00 AM	30	41	14	27	33	36	181							2:00 AM	23	30	10	19	24	27	133					
3:00 AM	19	41	18	22	13	23	136							3:00 AM	15	30	13	16	10	17	101					
4:00 AM	28	80	31	29	23	24	215							4:00 AM	21	59	23	21	17	18	159					
5:00 AM	35	164	73	46	20	49	387							5:00 AM	27	121	54	33	15	36	286					
6:00 AM	91	537	223	123	34	86	1094							6:00 AM	70	395	164	87	25	63	804					
7:00 AM	231	1187	417	230	165	142	2372							7:00 AM	177	874	307	163	121	105	1747					
8:00 AM	370	1315	556	250	282	222	2995							8:00 AM	283	968	409	177	208	163	2208					
9:00 AM	299	670	461	249	195	203	2077							9:00 AM	229	493	339	176	144	149	1530					
10:00 AM	248	479	424	278	229	166	1824							10:00 AM	190	353	312	197	169	122	1343					
11:00 AM	259	503	444	266	261	194	1927							11:00 AM	198	370	327	188	192	143	1418					
12:00 PM	280	421	476	261	281	168	1887							12:00 PM	214	310	350	185	207	124	1390					
1:00 PM	334	450	506	266	295	212	2063							1:00 PM	255	331	372	188	217	156	1519					
2:00 PM	307	500	468	294	322	212	2103							2:00 PM	235	368	345	208	237	156	1549					
3:00 PM	398	553	537	306	375	191	2360							3:00 PM	304	407	395	217	276	141	1740					
4:00 PM	530	598	685	389	465	305	2972							4:00 PM	405	440	504	275	342	225	2191					
5:00 PM	571	530	725	381	523	258	2988							5:00 PM	437	390	534	270	385	190	2206					
6:00 PM	475	528	542	406	447	223	2621							6:00 PM	363	389	399	287	329	164	1931					
7:00 PM	400	412	469	329	380	164	2154							7:00 PM	306	303	345	233	280	121	1588					
8:00 PM	262	273	338	247	258	126	1504							8:00 PM	200	201	249	175	190	93	1108					
9:00 PM	242	235	273	203	194	125	1272							9:00 PM	185	173	201	144	143	92	938					
10:00 PM	146	165	166	147	142	81	847							10:00 PM	112	121	122	104	105	60	624					
11:00 PM	76	144	104	85	90	65	564							11:00 PM	58	106	77	60	66	48	415					
12:00 AM	104	96	54	63	83	105	505							12:00 AM	80	71	40	45	61	77	374					
24-Hour	5788	10016	8032	4938	5147	3444	37365							24-Hour	4428	7372	5912	3497	3790	2537	27536					
6:00a- 8:30a	751	2837	1204	605	545	466	6406							6:00a- 8:30a	575	2089	886	428	401	343	4720					
3:30p - 6:00p	1311	1357	1610	982	1203	634	7095							3:30p - 6:00p	1003	999	1185	695	885	467	5233					
LPS- 5Hr	2062	4194	2814	1587	1748	1100	13501							LPS- 5Hr	1578	3088	2071	1123	1286	810	9953					
% of 24-Hr	35.6%	41.9%	35.0%	32.1%	34.0%	31.9%	36.1%							% of 24-Hr	35.6%	41.9%	35.0%	32.1%	33.9%	31.9%	36.1%					
Remain-19Hr	3726	5822	5218	3351	3399	2344	23864							Remain-19Hr	2850	4284	3841	2374	2504	1727	17583					
% of 24-Hr	64.4%	58.1%	65.0%	67.9%	66.0%	68.1%	63.9%							% of 24-Hr	64.4%	58.1%	65.0%	67.9%	66.1%	68.1%	63.9%					

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Table A.22 : Calculation of 24-Hour Inbound Heavy Vehicles

Unadjusted INDOT Mechanical Vehicle Counts										Axle Adjusted INDOT Mechanical Counts : Inbound Heavy Vehicles																				
Hour Ending	LPS 1 in					LPS 2 in					LPS 3 in					LPS 4 in					LPS 5 in					LPS 6 in				
	60 SB	41 EB off	70 EB	10 NB	43 WB off	Total IN	60 SB	41 EB off	70 EB	10 NB	43 WB off	Total IN	60 SB	41 EB off	70 EB	10 NB	43 WB off	Total IN	60 SB	41 EB off	70 EB	10 NB	43 WB off	Total IN	60 SB	41 EB off	70 EB	10 NB	43 WB off	Total IN
1:00 AM	43	87	42	41	67	40	320	1:00 AM	5	10	5	5	5	8	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5	5
2:00 AM	21	72	35	22	15	15	180	2:00 AM	2	8	4	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2
3:00 AM	25	40	14	22	23	27	151	3:00 AM	3	5	2	2	2	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
4:00 AM	27	58	23	24	38	19	189	4:00 AM	3	7	3	3	3	4	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2
5:00 AM	67	98	28	56	64	85	398	5:00 AM	8	11	3	6	7	10	45	3	6	7	10	45	3	6	7	10	45	3	6	7	10	45
6:00 AM	245	195	123	170	206	173	1112	6:00 AM	28	22	14	19	23	20	126	19	23	20	126	19	23	20	126	19	23	20	126	19	23	20
7:00 AM	448	357	397	322	466	235	2225	7:00 AM	51	41	45	36	53	27	253	41	45	27	253	41	45	27	253	41	45	27	253	41	45	27
8:00 AM	565	460	570	435	512	216	2758	8:00 AM	64	52	65	49	58	25	313	52	65	25	313	52	65	25	313	52	65	25	313	52	65	25
9:00 AM	368	448	462	292	425	154	2149	9:00 AM	42	51	53	33	48	18	245	42	53	18	245	42	53	18	245	42	53	18	245	42	53	18
10:00 AM	307	397	405	274	346	173	1902	10:00 AM	35	45	46	31	39	20	216	35	46	20	216	31	39	20	216	31	39	20	216	31	39	20
11:00 AM	289	366	415	252	353	176	1851	11:00 AM	33	42	47	29	40	20	211	33	47	20	211	31	40	20	211	31	40	20	211	31	40	20
12:00 PM	274	434	422	252	365	186	1933	12:00 PM	31	49	48	29	41	21	219	31	49	21	219	29	41	21	219	29	41	21	219	29	41	21
1:00 PM	286	452	508	274	386	159	2065	1:00 PM	33	51	58	31	44	18	235	33	51	18	235	31	44	18	235	31	44	18	235	31	44	18
2:00 PM	302	479	569	280	399	203	2232	2:00 PM	34	54	65	32	45	23	253	34	65	23	253	32	45	23	253	32	45	23	253	32	45	23
3:00 PM	380	611	593	306	405	201	2496	3:00 PM	43	69	67	35	46	23	283	43	69	23	283	35	46	23	283	35	46	23	283	35	46	23
4:00 PM	381	809	692	308	390	187	2767	4:00 PM	43	92	79	35	44	21	314	43	92	21	314	35	44	21	314	35	44	21	314	35	44	21
5:00 PM	372	1027	799	323	432	193	3146	5:00 PM	42	117	500	91	37	49	22	358	42	117	22	358	91	37	49	22	358	91	37	49	22	358
6:00 PM	392	1073	841	350	327	224	3207	6:00 PM	45	122	96	40	37	25	365	45	122	25	365	40	37	25	365	40	37	25	365	40	37	25
7:00 PM	281	694	610	263	352	122	2322	7:00 PM	32	79	69	30	40	14	264	32	79	14	264	69	30	40	14	264	69	30	40	14	264	
8:00 PM	228	449	393	195	254	110	1629	8:00 PM	26	51	45	22	29	13	186	26	51	13	186	45	22	29	13	186	45	22	29	13	186	
9:00 PM	188	331	358	141	182	129	1329	9:00 PM	21	38	41	16	21	15	186	21	38	15	186	41	16	21	15	186	41	16	21	15	186	
10:00 PM	127	315	258	108	158	91	1057	10:00 PM	14	36	29	12	18	10	119	14	36	10	119	29	12	18	10	119	29	12	18	10	119	
11:00 PM	94	192	108	75	87	96	652	11:00 PM	11	22	12	8	10	11	74	11	22	8	10	11	12	8	10	11	12	8	10	11	74	
12:00 AM	42	133	84	49	40	30	378	12:00 AM	5	15	10	6	5	3	44	5	15	3	44	10	6	5	3	44	10	6	5	3	44	
24-Hour	5752	9577	8749	4834	6292	3244	38448	24-Hour	654	1089	997	548	714	371	4373	654	1089	371	4373	997	548	714	371	4373	997	548	714	371	4373	
11:00a-1:30p	1197	1041	1198	903	1191	528	6058	11:00a-1:30p	81	127	139	76	108	51	581	81	127	51	581	139	76	108	51	581	139	76	108	51	581	
LPS: 2.5Hr	955	2505	1986	827	954	511	7737	LPS: 2.5Hr	81	127	139	76	108	51	581	81	127	51	581	139	76	108	51	581	139	76	108	51	581	
% of 24-Hr	2152	3546	3184	1730	2145	1039	13795	% of 24-Hr	12.4%	11.7%	13.9%	13.9%	15.1%	13.3%	13.3%	12.4%	11.7%	13.3%	13.3%	13.9%	13.9%	15.1%	13.7%	13.3%	13.9%	13.9%	15.1%	13.7%	13.3%	
Remain-19Hr	3600	6031	5565	3104	4147	2205	24653	Remain-19Hr	573	962	858	472	606	320	3792	573	962	320	3792	858	472	606	320	3792	858	472	606	320	3792	
% of 24-Hr	62.6%	63.0%	63.6%	64.2%	65.9%	68.0%	64.1%	% of 24-Hr	87.6%	88.3%	86.1%	86.1%	84.9%	86.7%	86.7%	87.6%	88.3%	86.1%	86.7%	86.1%	84.9%	86.3%	86.3%	86.7%	86.3%	86.7%	86.3%	86.7%		
Independent Calculation for Trucks :										Independent Calculation for Trucks :																				
Axle Corr. Fact.					Axle Corr. Fact.					Axle Corr. Fact.					Axle Corr. Fact.															
16.1%					16.1%					16.1%					16.1%															
15.7%					15.7%					15.7%					15.7%															



Expansion of Through Trip Matrices Using 24-Hour Vehicle Estimates

The final procedure in the estimation of daily through trips is the expansion of the through trip matrices [Tables A.05 and A.08] from AM, PM and Mid-Day survey period to full 24-hour weekday matrices. This procedure employs the patterns of through trip distribution established separately for passenger vehicles and heavy vehicles and uses the relationships between through trip volumes and total trip volumes to guide the expansion calculations.

Because through trips made by passenger vehicles and heavy vehicle were shown to exhibit different distribution patterns, it is necessary to calculate the 24-hour through trip matrices separately by vehicle type. These matrices can then be added together for estimated total through trip distribution.

The final adjusted through trip values calculated in Tables A.18 and A.19 were applied to the preliminary through trip matrices shown in Tables A.05 and A.08. [ The mechanics of this expansion are the same as those described in detail for Tables A.04 and A.05. ] After this adjustment, the 2.5-hour AM and PM passenger vehicle matrices were added together to give a 5-hour passenger vehicle through trip matrix. Completion of the expansion procedure will be described in detail for passenger vehicles using examples from Table A.24.

The procedure for heavy vehicle expansion is identical, except for the use of 2.5-hour sample data for the structure of the matrix. Table A.25 shows the spreadsheet calculations for daily heavy vehicle through trips.

The 5-hour passenger vehicle through trip matrix was expanded to 24-hours by a series of spreadsheet calculations shown in Table A.24. Row and column totals of the 5-hour through trip matrix represent the total number of through trips passing through inbound [1-6] and outbound [A-F]; there are 1149 inbound trips and 1149 outbound trips. 5-hour total passenger vehicles at each station are taken from Tables A.20 [inbound] and A.21 [outbound]. Percentages associated with the 5-hour matrix are total through passenger vehicles divided by total passenger vehicles at each station. Subject to slight rounding differences, the expansion procedure will hold these percentages constant.

The 24-hour passenger vehicle through trip matrix, shown in the lower half of Table A.24, uses the cell values, totals, and reference data from the 5-hour matrix. Row and column expanded control totals equal the corresponding [Percentage] of 5-Hour Passenger Vehicles multiplied by [Adjusted 24-Hour Total Passenger Vehicles]; for example :

$$\text{Row 1 : } 738 = 17.3\% \times 4269$$

Summed row and column control totals equal 3179 total inbound trips and 3182 outbound trips.





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Adjusted 24-Hour Total Passenger Vehicle values are taken from Tables A.20 [inbound] and A.21 [outbound].

Each 24-hour cell value equals the average of two calculations. The corresponding 5-hour cell divided by its row total and multiplied by the 24-hour cell's row expanded control total and the 5-hour cell divided by its column total and multiplied by the 24-hour cell's column expanded control total; for example :

$$B1, \text{ Row 1 : } ( 180 / 276 ) \times 738 = 481$$

$$B1, \text{ Col B : } ( 180 / 282 ) \times 673 = 430$$

$$B1 \text{ in 24-hour matrix} = \text{average of } (481, 430) = 455$$

After all 24-hour cells are thus calculated, the resulting matrix retains the proportional relationship to 24-hour total inbound and outbound passenger vehicles as exhibited by the 5-hour survey sample. Summed row and column totals equal 3183 inbound and 3183 outbound trips. These row and column sums, or 24-Hour Through Passenger Vehicles, when divided by row or column 24-Hour Total Passenger Vehicles, yield equivalent percentages to the corresponding calculation of 5-hour volumes; for example :

$$\text{Row 1, 24-Hour : } ( 730 / 4269 ) = 17.1\%$$

$$\text{Row 1, 5-Hour : } ( 276 / 1597 ) = 17.3\% \text{ [check];}$$

$$\text{Column B, 24-Hour : } ( 722 / 5912 ) = 9.8\%$$

$$\text{Column B, 5-Hour : } ( 282 / 3088 ) = 9.1\% \text{ [check].}$$

The outermost band of calculations in the 24-hour passenger vehicle matrix shows 24-Hour Total Vehicles – both passenger and heavy – at each station. Percentages expressed are 24-Hour Through Passenger Vehicles divided by 24-hour Total Vehicles; for example :

$$\text{Row 1} = \text{Station 1 : } 4269 \text{ [Table A.20]} + 654 \text{ [Table A.22]} = 4923$$

$$730 / 4923 = 14.8\%$$

$$\text{Col A} = \text{Station A : } 4428 \text{ [Table A.21]} + 656 \text{ [Table A.23]} = 5084;$$

$$1094 / 5084 = 21.5\%$$

The same 24-hour Total Vehicle values are similarly used to derive the same statistical measure for each station in the heavy vehicle through trip matrix.



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Table A.24 : Derivation of Daily Passenger Vehicle Through Trips

## 5-Hour Through Passenger Vehicle Trip Matrix

	A	B	C	D	E	F	Total	Adjusted 5-Hr Tot. Pass. Veh.	Pct. of 5-Hr Pass. Veh.
1	180	23	41	9	23		276	1597	17.3%
2	227	18	25	29			299	2652	11.3%
3	23	16	16	121	8		184	2381	7.7%
4	90	44	18	11	11		174	1301	13.4%
5	18	42	96	12	3		171	1603	10.7%
6	34	7	2	2			45	776	5.8%
<b>Total</b>	<b>392</b>	<b>282</b>	<b>162</b>	<b>96</b>	<b>172</b>	<b>45</b>	<b>1149</b>	<b>10310</b>	
<b>Adjusted 5-Hr Total Pass. Veh.</b>	<b>1578</b>	<b>3088</b>	<b>2071</b>	<b>1123</b>	<b>1286</b>	<b>810</b>	<b>1149</b>		
<b>Pct. of 5-Hr Pass. Veh.</b>	<b>24.8%</b>	<b>9.1%</b>	<b>7.8%</b>	<b>8.5%</b>	<b>13.4%</b>	<b>5.6%</b>			

## 24-Hour Through Passenger Vehicle Trip Matrix

	A	B	C	D	E	F	Total	Adjusted 24-Hr Tot. Pass. Veh.	24-Hr Through Pass. Veh.	Pct. of 24-Hr Pass. Veh.	Adjusted 24-Hr Total. Veh.	Pct. of 24-Hr Tot. Veh.
1	455	64	119	25	67		738	4269	730	17.1%	4923	14.8%
2	625	50	73	82			807	7159	830	11.6%	8248	10.1%
3	64	41	47	344	24		505	6539	520	8.0%	7536	6.9%
4	252	114	51	32	33		486	3634	482	13.3%	4182	11.5%
5	52	112	278	36	9		502	4702	487	10.4%	5416	9.0%
6	101	21	6	6			141	2423	134	5.5%	2794	4.8%
<b>Expanded Control Total</b>	<b>1100</b>	<b>673</b>	<b>462</b>	<b>299</b>	<b>507</b>	<b>141</b>	<b>3182</b>	<b>28726</b>	<b>3183</b>			<b>33099</b>
<b>Adjusted 24-Hr Total Pass. Veh.</b>	<b>4428</b>	<b>7372</b>	<b>5912</b>	<b>3497</b>	<b>3790</b>	<b>2537</b>	<b>27536</b>					
<b>24-Hr Through Passenger Veh.</b>	<b>1094</b>	<b>722</b>	<b>464</b>	<b>281</b>	<b>489</b>	<b>133</b>	<b>3183</b>					
<b>Pct. of 24-Hr Pass. Veh.</b>	<b>24.7%</b>	<b>9.8%</b>	<b>7.8%</b>	<b>8.0%</b>	<b>12.9%</b>	<b>5.2%</b>						
<b>Adjusted 24-Hr Total . Veh.</b>	<b>5084</b>	<b>8612</b>	<b>6910</b>	<b>4154</b>	<b>4427</b>	<b>2964</b>	<b>32151</b>					
<b>Pct. of 24-Hr Total Veh.</b>	<b>21.5%</b>	<b>8.4%</b>	<b>6.7%</b>	<b>6.8%</b>	<b>11.0%</b>	<b>4.5%</b>						

Table A.25 : Derivation of Daily Heavy Vehicle Through Trips

## 2.5-Hour Mid-Day Through Heavy Vehicle Trip Matrix

	A	B	C	D	E	F	Total	Adjusted 2.5-Hr Tot. Heavy Veh.	Pct.of 2.5-Hr Heavy Veh.
1		6		1	16	0	24	81	29.6%
2	8		4	3	6		21	127	16.5%
3	2	1		2	1	0	6	139	4.3%
4	21	4	1		3	2	31	76	40.8%
5	4	5	9	5		0	23	108	21.3%
6	7		0	11	0		18	51	35.3%
<b>Total</b>	<b>42</b>	<b>16</b>	<b>15</b>	<b>37</b>	<b>10</b>	<b>3</b>	<b>123</b>	<b>582</b>	
<b>Adjusted 2.5-Hr Total Heavy Veh.</b>	<b>88</b>	<b>139</b>	<b>151</b>	<b>90</b>	<b>92</b>	<b>60</b>	<b>620</b>		
<b>Pct.of 2.5-Hr Heavy Veh.</b>	<b>47.7%</b>	<b>11.5%</b>	<b>9.9%</b>	<b>41.1%</b>	<b>10.9%</b>	<b>5.0%</b>			

## 24-Hour Through Heavy Vehicle Trip Matrix

	A	B	C	D	E	F	Total	Adjusted 24-Hr Total Heavy Veh.	24-Hr Through Heavy Veh.	Pct.of 24-Hr Heavy Veh.	Adjusted 24-Hr Total Veh.	Pct.of 24-Hr Total Veh.
1		51	7	123	0	8	194	654	189	28.9%	4923	3.8%
2	64		30	24	46		180	1089	164	15.1%	8248	2.0%
3	15	8		14	7	0	43	997	44	4.4%	7536	0.6%
4	154	32	7		21	14	224	548	228	41.6%	4182	5.5%
5	28	39	59	35		0	152	714	161	22.5%	5416	3.0%
6	52		0	80	0		131	371	132	35.6%	2794	4.7%
<b>Expanded Control Total</b>	<b>313</b>	<b>143</b>	<b>99</b>	<b>270</b>	<b>69</b>	<b>21</b>	<b>915</b>	<b>4373</b>	<b>918</b>			<b>33099</b>
<b>Adjusted 24-Hr Total Heavy Veh.</b>	<b>656</b>	<b>1240</b>	<b>998</b>	<b>657</b>	<b>637</b>	<b>427</b>	<b>4615</b>					
<b>24-Hr Through Heavy Veh.</b>	<b>313</b>	<b>130</b>	<b>103</b>	<b>276</b>	<b>74</b>	<b>22</b>						
<b>Pct.of 24-Hr Heavy Veh.</b>	<b>47.7%</b>	<b>10.5%</b>	<b>10.3%</b>	<b>42.0%</b>	<b>11.6%</b>	<b>5.2%</b>	<b>918</b>					
<b>Adjusted 24-Hr Total Veh.</b>	<b>5084</b>	<b>8612</b>	<b>6910</b>	<b>4154</b>	<b>4427</b>	<b>2964</b>						
<b>Pct.of 24-Hr Total Veh.</b>	<b>6.2%</b>	<b>1.5%</b>	<b>1.5%</b>	<b>6.6%</b>	<b>1.7%</b>	<b>0.7%</b>	<b>32151</b>					

## Evaluation of Daily Through Trip Estimates

Estimates of daily through trips by two broad types, passenger and heavy vehicles, are combined in Table A.26 to show total daily through trips. Row and column sums for 4101 inbound and 4101 outbound daily through trips are also compared to the daily total vehicles at each station.

Examination of Table A.26 supports several observations about the characteristics of through trips using the SR 9 corridor. The heaviest volumes of through trips appear to move between I-70 west of Greenfield and SR 9 north of its interchange with I-70. Such trips impact only the northern fringe of Greenfield. The second heaviest volumes of through trips are those using SR 9 to traverse Greenfield between its northern and southern limits. All through trips using these two origin-destination combinations pass through the I-70 / SR 9 interchange and are primarily passenger vehicles.

A third, but less significant, volume of through trips cross Greenfield on US 40 between its western and eastern limits. On this origin-destination combination, the number of heavy vehicle through trips is nearly equal to the number of passenger vehicle through trips. Although these trips appear to be independent of through trip movements originating on I-70 or SR 9, they compete with the north-south through trips at the intersection of US 40 and SR 9.

The congestive impact of converging through trips at the intersection of US 40 and SR 9 and along the SR 9 corridor between the I-70 interchange and US 40 should be carefully considered. Special attention must also be given to the presumed volume of external-internal and internal-external trips using the SR 9 corridor. Although by definition these are not through trips, having either destination or origin at some location within Greenfield, such trips share the same routes carrying the heaviest volumes of through trips.

### Comparison of the Daily Through Trip Matrix with the INDOT Sub-Area Analysis

The INDOT SR 9 External Travel Analysis was conducted as a sub-area analysis using the TransCAD statewide model. This analysis was prepared on July 27, 2000 using a year 1995 trip table and a base highway network for the U.S. Census-defined Greenfield Urban Area. A brief summary of the analysis is quoted :

"... 1995 through trips assigned to SR 9 = 8468 x model under assignment factor of 1.19 results in 10,077 estimated through trips eligible for bypass (approximately 50% of travel on SR 9 in 1995). Of these 5,047 through trips are associated with I-70 and use the SR 9 interchange ( $5,047 \times 1.19 = 6,006$  or approximately 60% of the through trips)."





## SR 9 Environmental Assessment / Corridor Study

Table A.27 shows the matrix of through trips calculated for the SR 9 corridor study compared to the earlier INDOT estimates fitted to the same directional matrix structure.

There are six cells in the INDOT matrix corresponding to movements not modeled by the INDOT sub-area analysis.

There is a substantial difference between the two estimates. The INDOT estimate of 21,518 through trip movements must be reconciled against the 4,101 through trip movements found by expansion of the license plate survey. Plausible explanations for the apparent inconsistency are underscored by the differences in methodologies used to develop the two estimates.

Table A.26 : Estimated Daily Through Vehicle Trips

24-Hour Through Passenger Vehicle Trip Matrix							Total Through Passenger Vehicles	24-Hr Total Vehicles	Pct. of 24-Hr Tot. Veh.
	A	B	C	D	E	F			
1		455	64	119	25	67	730	4923	14.8%
2	625		50	73	82		830	8248	10.1%
3	64	41		47	344	24	520	7536	6.9%
4	252	114	51		32	33	482	4182	11.5%
5	52	112	278	36		9	487	5416	9.0%
6	101		21	6	6		134	2794	4.8%
Total Through Passenger Vehicles	1094	722	464	281	489	133	3183		
24-Hr Total Vehicles	5084	8612	6910	4154	4427	2964	3183		
Pct. of 24-Hr Total Veh.	21.5%	8.4%	6.7%	6.8%	11.0%	4.5%			

24-Hour Through Heavy Vehicle Trip Matrix							Total Through Heavy Vehicles	24-Hr Total Vehicles	Pct. of 24-Hr Tot. Veh.
	A	B	C	D	E	F			
1		51	7	123	0	8	189	4923	3.8%
2	64		30	24	46		164	8248	2.0%
3	15	8		14	7	0	44	7536	0.6%
4	154	32	7		21	14	228	4182	5.5%
5	28	39	59	35		0	161	5416	3.0%
6	52		0	80	0		132	2794	4.7%
Total Through Heavy Vehicles	313	130	103	276	74	22	918		
24-Hr Total Vehicles	5084	8612	6910	4154	4427	2964	918		
Pct. of 24-Hr Total Veh.	6.2%	1.5%	1.5%	6.6%	1.7%	0.7%			

24-Hour Total Through Trip Matrix							Total Through Vehicles	24-Hr Total Vehicles	Pct. of 24-Hr Tot. Veh.
	A	B	C	D	E	F			
1		506	71	242	25	75	919	4923	18.7%
2	689		80	97	128		994	8248	12.1%
3	79	49		61	351	24	564	7536	7.5%
4	406	146	58		53	47	710	4182	17.0%
5	80	151	337	71		9	648	5416	12.0%
6	153		21	86	6		266	2794	9.5%
Total Through Vehicles	1407	852	567	557	563	155	4101		
24-Hr Total Vehicles	5084	8612	6910	4154	4427	2964	4101		
Pct. of 24-Hr Total Veh.	27.7%	9.9%	8.2%	13.4%	12.7%	5.2%			

## SR 9 Environmental Assessment / Corridor Study

Table A.27 : Comparison of Daily Through Trip Estimates : O-D Survey vs INDOT Analysis

24-Hour Total Through Trip Matrix

	A	B	C	D	E	F	Total Through Vehicles	24-Hr Total Vehicles	Pct. of 24-Hr Tot. Veh.
1		506	71	242	25	75	919	4923	18.7%
2	689		80	97	128		994	8248	12.1%
3	79	49		61	351	24	564	7536	7.5%
4	406	146	58		53	47	710	4182	17.0%
5	80	151	337	71		9	648	5416	12.0%
6	153		21	86	6		266	2794	9.5%
Total Through Vehicles	1407	852	567	557	563	155	4101		
24-Hr Total Vehicles	5084	8612	6910	4154	4427	2964			
Pct. of 24-Hr Total Veh.	27.7%	9.9%	8.2%	13.4%	12.7%	5.2%			

24-Hour Total Through Trip Matrix [INDOT Model Subarea]

	A	B	C	D	E	F	Total Through Vehicles	24-Hr Total Vehicles	Pct. of 24-Hr Tot. Veh.
1		1323	140	1538	89		3090	4923	62.8%
2	1435			474	773		2682	8248	32.5%
3	59			277	5037	169	5542	7536	73.5%
4	1409	711	513		460	196	3289	4182	78.6%
5	186	2041	3348	657			6232	5416	115.1%
6			375	308			683	2794	24.4%
	3089	4075	4376	3254	6359	365	21518		
24-Hr. Total	5084	8612	6910	4154	4427	2964			
	60.8%	47.3%	63.3%	78.3%	143.6%	12.3%			

The PKG/PIC O-D survey counted and expanded a finite number of through trips strictly constrained by identification and station to station travel time parameters. Therefore, although based on a sampling technique, this estimate is biased toward the minimum number of actual weekday through trips.

The INDOT estimate is based on a mathematical model using a road network suitable for statewide analysis. Adapting the relatively low density of the statewide road network to a small geographic sub-area tends to force multiple all-or-nothing travel assignments to the limited number of physical links. This means that numerous trips that would physically use dispersed county or local roads are concentrated by the model onto the SR 9 and US 40 roadway links. Furthermore, the distinctions between through trips and near through trips ( actually external-internal and internal-external trips) are much more difficult to quantify at the geographic resolution of the statewide model in a county exhibiting rural characteristics, such as those surrounding the Greenfield urban area. Therefore, the INDOT estimate should be viewed as biased toward a maximize number of potential weekday through trips.



## SR 9 Environmental Assessment / Corridor Study

Table A.28 is intended to complete the reconciliation between the two estimates by illustrating the numeric relationships between the individual cells in each matrix and by comparing these relationships against each other.

Matrix [A] shows the percentage of each cell to the sum of through trips in the matrix based on expansion of the license plate survey and mechanical vehicle counts. Row (inbound) and column (outbound) averages are displayed and also compared to the 4.1% overall average for the entire matrix. Note that the six through trip movements not modeled by the INDOT analysis are excluded from the survey matrix to enable direct comparisons.

Table A.28 : Statistical Comparison of O-D Survey and INDOT Analysis Through Trip Results

[ A ] Compared Relative Percentages [License Plate Survey]

	A	B	C	D	E	F	Inbound Row Average	24-Hr Overall Average	Inbound Pct. of Overall
1		12.3%	1.7%	5.9%	0.6%		5.1%	4.1%	124.5%
2	16.8%			2.4%	3.1%		7.4%	4.1%	179.7%
3	1.9%			1.5%	8.6%	0.6%	3.1%	4.1%	76.0%
4	9.9%	3.6%	1.4%		1.3%	1.1%	3.5%	4.1%	83.8%
5	2.0%	3.7%	8.2%	1.7%			3.9%	4.1%	94.2%
6			0.5%	2.1%			1.3%	4.1%	31.6%
Outbound Col Average	7.6%	6.5%	3.0%	2.7%	3.4%	0.9%	4.0%	4.1%	98.3%
24-Hr Overall Average	4.1%	4.1%	4.1%	4.1%	4.1%	4.1%			
Outbound Pct. of Overall	185.0%	157.9%	71.8%	65.7%	82.2%	20.9%	97.3%		

[ B ] Compared Relative Percentages [INDOT Model Subarea]

	A	B	C	D	E	F	Inbound Row Average	24-Hr Overall Average	Inbound Pct. of Overall
1		6.1%	0.7%	7.1%	0.4%		3.6%	4.5%	79.0%
2	6.7%			2.2%	3.6%		4.2%	4.5%	91.4%
3	0.3%			1.3%	23.4%	0.8%	6.4%	4.5%	141.7%
4	6.5%	3.3%	2.4%		2.1%	0.9%	3.1%	4.5%	67.3%
5	0.9%	9.5%	15.6%	3.1%			7.2%	4.5%	159.3%
6			1.7%	1.4%			1.6%	4.5%	34.9%
Outbound Col Average	3.6%	6.3%	5.1%	3.0%	7.4%	0.8%	4.4%	4.3%	95.6%
24-Hr Overall Average	4.5%	4.5%	4.5%	4.5%	4.5%	4.5%			
Outbound Pct. of Overall	79.0%	138.9%	111.9%	66.5%	162.5%	18.7%	96.2%		

[ A ] / [ B ] Ratio of Percentage Comparisons

	A	B	C	D	E	F	Inbound Row Average	24-Hr Overall Average	Inbound Pct. of Overall
1		2.01	2.66	0.83	1.47		1.74	1.42	122.6%
2	2.52			1.07	0.87		1.49	1.42	104.7%
3	7.03			1.16	0.37	0.75	2.32	1.42	163.5%
4	1.51	1.08	0.59		0.60	1.26	1.01	1.42	71.0%
5	2.26	0.39	0.53	0.57			0.94	1.42	65.8%
6			0.29	1.47			0.88	1.42	61.9%
Outbound Col Average	3.33	1.16	1.02	1.02	0.83	1.00	1.39	1.40	98.2%
24-Hr Overall Average	1.42	1.42	1.42	1.42	1.42	1.42			
Outbound Pct. of Overall	234.2%	81.4%	71.7%	71.6%	58.3%	70.5%	98.0%		



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Matrix [B] presents the same cell and matrix statistics for through trips developed by the INDOT sub-area analysis. Row (inbound) and column (outbound) averages are displayed and also compared to the 4.5% overall average for the entire matrix.

Numeric differences between matrix [A] and matrix [B] are presented in the third matrix by expressing the differences as the ratio of [A] values divided by corresponding [B] values. This calculation reveals a relative similarity between [A] and [B] values when the magnitude of variance between individual cell ratios and the overall [A]/[B] ratio of 1.42 is evaluated.

Four cells, or 18% of twenty-two cells in the matrix, are equal or greater than 2.13 [ $1.42 + 50\% \times (1.42)$ ]. Seven (7) cells, or 32% of the cells in the matrix are equal or less than 0.71 [ $1.42 - 50\% \times (1.42)$ ]. The remaining eleven (11) cells, or 50%, are within plus or minus half the value of the overall ratio of 1.42. Furthermore, cells exhibiting the greatest variance correspond to matrix [A] or matrix [B] cells with small values.

Table A.28 supports the contention that while there are substantial differences in overall through trip *volumes*, as developed by the two different methodologies, there is an equally substantial similarity in the *distribution* of through trips regardless of method. This conclusion suggests that the magnitude of through trips using the SR 9 corridor are probably in the intermediate range between the PKG/PIC survey values and those generated by the INDOT sub-area analysis.

Accordingly, volumes and distribution of through trips must be carefully weighed when the conflict between through trips and external-internal trip ends is incorporated into the justification analysis for a SR 9 bypass in the vicinity of Greenfield.

